# ESTIMATION OF SPECIFIC FLOW DURATION CURVES USING BASIN CHARACTERISTICS OF RIVERS IN SOLAKLI AND KARADERE BASINS

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#### **ABSTRACT**

# ESTIMATION OF SPECIFIC FLOW DURATION CURVES USING BASIN CHARACTERISTICS OF RIVERS IN SOLAKLI AND KARADERE BASINS

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Demand for energy is constantly growing both in the world and in Turkey. Sustainable development being an important concept, development of small hydro power projects has been popular in recent years. Eastern Black Sea Basin in Turkey has a lot of small hydro power potential because of high amount of precipitation and existence of steep slopes. Since the amount of river runoff is the only parameter that is variable in order to determine the power potential, it is vital to estimate the project discharge in ungauged basins accurately that have hydro power potential. Projects discharges of hydro-power plants in ungauged basins have been calculated using conventional methods up to now. This study aims to introduce a statistical model in linear and multi-variate form using the topographical and morphological parameters derived from GIS and hydro-meteorological variables to estimate the specific flow duration curves of potential small hydro-power locations for the selected study areas in Eastern Black Sea Region namely Solaklı and Karadere basins. As well as developing an annual

regression model using the annual values of hydro-meteorological parameters; seasonal regression model (spring season) has also been developed by including the mean seasonal (spring) air temperature variable instead of snow covered area (SCA) in addition to basin parameters. By studying the spring model, effect of different variables from the annual model were tested and discussed with some recommendations for the future studies.

**Keywords:** Ungauged Basin, Small Hydro-Power, Statistical Model, GIS, Eastern Black Sea Basin

# SOLAKLI VE KARADERE HAVZALARINDAKİ AKARSULARIN HAVZA KARAKTERISTİKLERİNİ KULLANARAK ÖZGÜL DEBİ SÜREKLİLİK EĞRİLERİNİN TAHMİN EDİLMESİ

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Dünyada ve Türkiye'de enerjiye olan talep gittikçe artmaktadır. Sürdürülebilir kalkınma konseptinin önemiyle beraber, küçük hidroelektrik santrallerin geliştirilmesi son yıllarda önem kazanmıştır. Bu bağlamda yüksek yağış oranı ve yüksek eğimlerin varlığı sebebiyle Türkiye'deki Doğu Karadeniz Bölgesi'nin ciddi bir hidroelektrik potansiyeli mevcuttur. Hidroelektrik potansiyel belirlenmesinde nehir akımları tek değişken olduğundan ötürü, hidroelektrik potansiyeli olan ve ölçüm istasyonu olmayan havzalardaki akım miktarını en iyi şekilde belirlemek çok önemlidir. Şimdiye dek; ölçüm istasyonu olmayan hidroelektrik santrallerinin havzalarının hesapları geleneksel metotlatla yapılagelmiştir. Bu çalışmada; Doğu Karadeniz Bölgesi'nde seçilmiş Karadere ve Solaklı havzalarındaki potansiyel küçük hidroelektrik santrallerinin özgül debi süreklilik eğrilerinin tahmin edilmesi için CBS yöntemleri kullanılarak çıkarılan topografik, morfolojik ve hidro-meteorolojik parametrelerle kurulan değiskenli istatistiki modellemelerin coklu gelistirilmesi amaçlamaktadır. Yıllık bazda hidro-meteorolojik parametrelerin yıllık değerleri kullanılarak bir regresyon modeli geliştirildiği gibi; mevsimsel

(ilkbahar) ortalama hava sıcaklığını karla kaplı alan parametresi yerine kullanarak, diğer havza parametrelerine ilaveten mevsimel (ilkbahar) model de geliştirilmiştir. İlkbahar modelini kurarak, farklı parametrelerin etkisi test edilmiş ve gelecekteki çalışmalar için önerilerde bulunulup sonuçlar tartışılmıştır.

**Anahtar Kelimeler:** Ölçüm İstasyonu Olmayan Havza, Küçük Hidroelektrik, İstatistiki Model, CBS, Doğu Karadeniz Havzası

To the Meaning of My Life...

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#### **CHAPTER 1**

#### INTRODUCTION

#### 1.1 Definition of the Problem

Demand for energy is constantly growing both in Turkey and in the world as long as the population goes on increasing and industries of nations keep growing. Supplying the necessary demand has not been sufficient since the term sustainable development was coined. This means that while meeting the demands of humanity, the future generations should also be able to benefit from the world's resources. Concordantly, the concept called "renewable energy" has been a very important issue. Sunlight, tides, wind, rain and water are some of the most important renewable energy sources that are being used in energy supply. The share of hydroelectricity in electricity generation is 15 %, the rest of renewable sources contributing only 3 % (Web 1). In this context, hydropower remains very important and has been gaining much importance in growing countries like Turkey in recent years.

The use of hydropower has always been important for the advance of civilization and it dates back to ancient ages. With the triggering effect of the industrial revolution, hydropower has significantly contributed to power generation. In 1881, Cragside House became the first house in the world to be lit using hydroelectric power. In 1882, world's first hydroelectric power

plant began operation on the Fox River in Appleton, Wisconsin. In the following years, hydropower generation continued to be essential for development.

In Turkey, during 1950s, the total amount of hydroelectricity used to consist of only 4.4 % of Turkey's whole electricity generation. By 2008, this ratio has risen to 17 %. Currently being only 35 % of the economically feasible hydropower is under operation, development of hydropower projects still remain important (Yuksel et al., 2008).

To generate electricity from hydro power, it was popular to construct large dams. Dams not only provide a reliable and a large amount of power supply; but also they are used for irrigation, water supply, flood control, recreational, etc. purposes. In recent years; the damages and negative impacts of dams like social, environmental and economical impacts have come up. A lot of people may be forced to leave their hometowns and a large amount of lands may be flooded. Furthermore, operation of dams goes on until the dead volume of the reservoir is filled with sediment. These problems have brought a new concept called "small hydro power".

Small hydro power is a development of hydro power on a small scale. Having a large amount of small hydropower potential in Turkey, investors have lately been interested in electricity generation from small hydro electric power plants. Eastern Black Sea Basin is the most important basin regarding with the small hydro power potential according to Yuksel et al. (2008).

Installed power capacity of a hydro power plant is a function of height difference between the source and outflow of water (head) and discharge. Since head is accepted to be constant, the only variable in power function is discharge. Conventional methods have been used to estimate project discharge and a fresh method has to be offered to estimate project discharges of small hydro electric power plants in the basins that are commercially popular.

In order to determine a project discharge of a certain project, the flow duration curve for annual period is used. However, flows in Turkey are not regular and the snowmelt contribution to the runoff is significant especially in Eastern Black Sea Region. Snowmelt season is the spring season and it is important to study not only annual flow duration curves, but also seasonal flow duration curves for the spring season. Therefore, to determine an extra installed capacity for the spring season is beneficiary for the investors which allow them to make use of high flows in the spring season resulting from snowmelt.

#### 1.2 Aim of the Study

The aim of this study is to estimate seasonal and annual flow duration curves for the range of probabilities between 5% and 40% by developing statistical models for the ungauged basins within Solaklı and Karadere watersheds (located in Eastern Black Sea Basin) in order to determine the project discharge values of potential hydropower locations.

The model parameters are both topographic and hydro-meteorological data where the topographic and morphological parameters are extracted by using Geographic Information System (GIS). Linear, relief, morphological and shape measures are the categories of topographic parameters. Furthermore, the snow covered area (SCA) data from satellite images of the selected basins are associated in the model by seeking a relationship with mean daily temperature values of the related meteorological stations. By setting up regression models using several statistical methods; the dominant parameters are defined and the flow values corresponding to 8 percentiles of flow duration curve (5%, 10%, 15%, 20%, 25%, 20%, 30%, 35% and 40%) are determined for both annual and seasonal models. The season here, refers to the spring season. Then the model is validated by comparing the results with the values of a selected flow gauging station.

### 1.3 Organization of the Thesis

This study is composed of 6 chapters. This chapter being the "Introduction" chapter, the other chapters are given as:

In Chapter 2, a brief literature review related with the scope of this thesis was provided.

In Chapter 3, the description of the study areas and also the information about the collected data as hydro-meteorological, topographic and snow covered area (SCA) data are given.

In Chapter 4, the analyses and processing of the collected data are explained. Completion of missing hydro-meteorological data, deriving the seasonal and annual flow duration curves of the stream flow gauging stations and the project sites, developing the Digital Elevation Model (DEM) of the basins within Solaklı and Karadere watersheds, delineation of the basins and deriving the topographic parameters and studying the relationship between SCA and mean daily temperature of the selected stations are given.

In Chapter 5, the development of multi-linear and multi-variate regression models, fitting the appropriate functions for FDCs and validation of the results are given.

Chapter 6 is the last chapter of thesis and final discussions, conclusions and recommendations are listed.

#### **CHAPTER 2**

#### LITERATURE REVIEW

Small hydroelectric power plants (Small HEPPs) are widely accepted to be the power plants that have less capacity than 10 MW (Paish, 2002). They have been popular in Turkey in recent years after the liberation of the energy market in 2001. Also small hydropower potential of Turkey is high because of being a mountainous country (Yuksel et al., 2008). Günyaktı et al. (2008) state that without expensive civil works, it is possible to develop high heads with relatively small discharges which can produce desired amount of energy. According to the study of Yuksel et al. (2008); 30.34 % of the hydropower energy will be generated from small HEPPs when the projects under design stage will be completed. Particularly, the result of a study show that 52.18 % of annual energy of all projects is in Eastern Black Sea Basin (Yuksel et al., 2008). According to Yuksel et al. (2008); the reasons behind high small hydropower potential in Eastern Black Sea Basin are being the wettest basin of Turkey as the annual total amount of precipitation in Rize goes up to 2329 mm and being covered with sharp valleys with steep slopes thus providing considerable heads and discharges.

Yanık et al. (2005) offer a method to obtain regional flow duration curves to determine the design discharge values of the regional hydroelectric potential in ungauged basins. Conventional methods like drainage area ratio methods

to determine the flow duration curves (FDC) of the ungauged basins do not seem to be sufficient in large spatial scales. Eastern Black Sea Basin where Solaklı and Karadere basins are located is selected for the case study. Considering the specific flow duration curves and the probability of exceedance interval between 30 % and 100 % (the required interval for project discharges) and also using the cluster analysis methods, Solaklı and Karadere basins came up to be in the same homogeneous region. Moreover, an analytical function is fitted to calculate the specific discharge corresponding to a given exceedance probability within the range of 30 % and 100 %. The equation is given below and it is valid for the basins within region A that results from study of Yanık et al. (2005).

$$Q_t = 0.0668 - 0.002174t + 0.0002843t^2 - 0.0000001317t^3$$
 (2.1)

Where t is the flow percentile that is desired (%) and  $Q_t$  is the specific discharge corresponding to desired flow percentile ( $m^3/s/km^2$ ).

Estimation of river runoff is one of the key works in the water resources applications. Mohamoud, (2008) in his study predicted flow duration curves and streamflow time series for ungauged catchment in the Mid-Atlantic Region, USA. Step-wise regression analysis is performed and the dominant climatic and landscape parameters are identified. The regional flow duration curves are also developed. Climate, geomorphologic and soil descriptors come out to be the dominant parameters that influence the hydrology of the selected regions. The constructed flow duration curves are then compared

with the sites that are not included in the model for verification purpose. Furthermore, the streamflow time series that are predicted are compared with the catchments calculated by drainage area ratio methods.

Feasibility of the projects is based on head and discharge. Uncertainty in discharge estimation in ungauged catchments directly affects the feasibility. Therefore, setting a runoff model especially in poorly gauged basins has been attracting attention. Algancı et al., (2008); develop a regression model for Solaklı Basin which is one of the basins studied in this thesis. They use remote sensing (RS) and geographical information systems (GIS) to derive a digital elevation model (DEM). With the combination of hydro-meteorological data; they set up a regression model using GIS environment and verified the results using a sub-basin within Solaklı Basin. Both linear and logarithmic regression models are used and it is seen that logarithmic models provide better results compared to linear models. The parameters involved in these equations are mean basin elevation, basin area and rainfall values.

Snowmelt runoff is an important input for runoff in mountainous regions. Snowmelt runoff in mountainous eastern part of Turkey constitutes 60-70 % of total runoff during spring and early summer seasons where temperatures start to rise (Şorman A. A., 2005). In the study area where Solaklı and Karadere basins lie, snowmelt is an important input for the model since the mean altitude of the region is relatively high. In the literature, Zaherpour et al. (in press) include snow water equivalent (SWE) in long-term forecasting of riverflow and find out that snowmelt is a significant parameter in Dez Basin in Iran. They use both SWE and snow covered area (SCA) data and the results

show that substituting SCA for SWE depths are acceptable. Besides, after determining all the parameters including temperature, rainfall, SWE and input flow, riverflow forecasting model is developed in one to six-month time steps (Zaherpour et al., in press). The forecasting is made for 1971-1977 period. On the other hand, for the 1990-1997 period where satellite images are available; the SCA data is used instead of SWE depths data in developing the forecasting model.

Precipitations are extremely variable, both spatially and temporally, and the knowledge of its areal mean is a prerequisite to any serious water balance computations (Valery et al., 2009). It is not possible to observe the mean areal precipitation. There is not a perfect areal estimation even in densely gauged experimental catchments and it is obviously worse in data-sparse mountainous regions (Valery et al., 2009). In the study of Valery et al. (2009), they present an attempt to "invert" the hydrological cycle and to use streamflow measurements to improve the knowledge of precipitation input in data-sparse mountainous regions. In other words, they utilize streamflow measurements in order to guess how much rain falls at higher elevations where no observations are made. In this paper, two data sets of 31 Swiss and 94 Swedish catchments and three simple long-term water balance formulas are used. A simple two-parameter correcting model to regionalize precipitation from the too sparse precipitation gauging network; the first parameter ( $\alpha$ ) aims to correct snow undercatch by precipitation gauges while the second one (β) targets the precipitation-elevation relationship (Valery et al., 2009). According to the results of this study; identification of precipitation-elevation (β) relationship is easier than that of snow undercatch  $(\alpha)$ .

Predicting streamflow time series or some hydrological indices (like specific percentiles of flow duration curves, mean annual flows, etc.) of ungauged or poorly gauged basins have always attracted attentions of scientists. In the study of Masih et al. (2010); a conceptual rainfall-runoff model (HBV) is used for streamflow simulations in a basin in Iran. There are four measures which are defined for hydrologic similarity between a catchment simulated by HBV model and a selected ungauged or poorly gauged catchment. These are; drainage area, spatial proximity, catchment characteristics and flow duration curve (FDC). FDCs could be established from some regionalization methods available in the literature (Masih et al., 2010). The aim of this paper is to check whether the parameters of a conceptual model of a gauged catchment could be transferred for simulating streamflows of an ungauged or poorly gauged basin or not. The results of this study show that the similarity measures which are drainage area, spatial proximity and catchment characteristics do not give satisfactory results. However, FDC provides the best results among all. By using a statistical criterion called relative root mean square error (RRMSE), the similarities of catchments can be analysed. Thus, catchment similarity based on FDCs provides a sound basis for transferring model parameters from gauged catchments to data limited catchments in the study area (Masih et al., 2010).

In the study of Li et al. (2010), a new regionalization approach called the "index model" and its application to predict flow duration curves in ungauged basins are presented. Each parameter in a hydrological predictive tool in ungauged catchments is estimated from a set of catchment characteristics and climatic variables by the index model. This model could also easily be interpreted for FDCs. In the selected catchments located in south east

Australia; climatic factors and some catchment characteristics like leaf area index, elevation and fraction of total native woody generation come out to be significant parameters. Furthermore, index model provides the most accurate predictions followed by linear regression among nearest neighbour and hydrological similarity techniques.

Post (2004), presents a different method for estimating flow duration curve (FDC) using logarithmic transformation. In this study FDC is defined using two parameters; the "cease to flow" point and the slope of FDC. This method is applied in a region in Australia (Burdekin catchment) and parameters are related to area, mean annual precipitation, drainage density and total stream length of the catchments. By this way, a regionalisation procedure is developed where FDC of an ungauged catchment could be estimated based on characteristics of the related catchment.

#### **CHAPTER 3**

#### DESCRIPTION OF THE STUDY AREA AND DATA COLLECTION

#### 3.1 Description of the Study Area

The study region which is composed of the watersheds of Solaklı and Karadere streams locates in the Eastern Black Sea Region. These two watersheds are named according to their main streams. Karadere Watershed lies between the coordinates; 40° 48′- 40° 95′ north latitudes and 39° 72′ - 40° 10′ east longitudes. Solaklı Watershed lies between the coordinates 40° 18′ - 40° 95′ north latitudes and 40° 10′ - 40° 48′ east longitudes. Both lie within the province of Trabzon. Solaklı and Karadere watersheds are adjacent and the locations of Solaklı and Karadere watersheds in Turkey are shown in Figure 3.1.

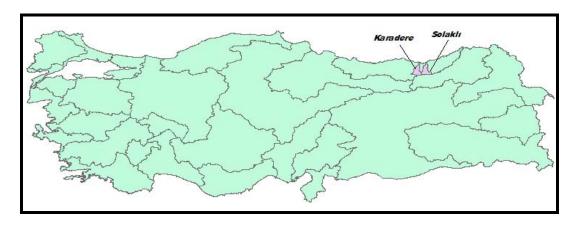


Figure 3.1 The locations of Karadere and Solaklı basins in Turkey

Karadere and Solaklı streams rise in the Horos Soğanlı and Haldizen mountains from about 2850 m and 3350 m respectively and flow into the Black Sea. The Kara Stream rises at the southeastern region of the basin and joins Alçak Stream at about 1600 m. Then the stream joins Karadere Stream at about 1410 m. For Solaklı Watershed; Haldizen Stream rises at southeastern region of the watershed and joins Solaklı Stream at about 300 m. The watershed boundaries and stream network are shown in Figure 3.2.

The areas of Karadere and Solaklı watersheds are 729.26 km² and 758.44 km² respectively. Solaklı Watershed is covered by 23% of coniferous forest, 20% of deciduous forest, 16% of bare land, 14% of pasture, 12% of rocky land and 12% of agricultural area (Alganci et al., 2010). Since the Karadere Watershed is adjacent to Solaklı Watershed; the land use distribution is expected to be almost same with that of Solaklı.

The main reason behind the selection of this project site is that there is a large amount of hydro-power potential and planned small HEPPs. Small HEPPs are going to contribute about 5% of all hydro-power energy when the small HEPPs under design stage are in operation. Furthermore, about more than 50 % of the small hydro-power energy potential is in the Eastern Black Sea region (Yüksel et al., 2008). Particularly, Solaklı and Karadere watersheds are two of the important streams in the region and are found to be in the same homogeneous region which is obtained as a result of the study of Yanık, et al. 2005.

#### 3.2 Data Collection

#### 3.2.1 Introduction

Hydrologic data are so important for hydrologic practices and science. These data are critical for performing risk assessment and economic analysis and also evaluating the impact of the water projects on public. Therefore, good, consistent historical data are essential for modeling to make accurate predictions (Web 2). The quality of data has a lot of importance also in scientific researches. Collection, arrangement and analysis of the hydrologic data are among the most time consuming activities in overall hydrologic studies (Zaherpour et al., in press). For collection, storage and analysis, most countries have one or more agencies responsible for the management of hydrologic data. In Turkey, State Meteorological Organization (DMI), State Hydraulic Works (DSI), Electrical Power Resources Survey and Development Administration (EIEI) and General Directorate of Rural Services (KHGM) collect and also analyze hydrologic and meteorologic data (Usul, 2001).

There are several types of data that are used in this study including hydrometeorological data such as daily rainfall, daily temperature and mean daily discharges; topographic data such as digital topographic map and snow covered area (SCA) of the study area. The rainfall and temperature data are gathered from DMI, the discharge data are gathered both from DSI and EIEI. Also the snow covered area maps are obtained through the result of SEVIRI image files (Surer, 2008). The input for the digital elevation model is gathered from DSI.

#### 3.2.2 Hydro-Meteorological Data

The meteorological stations in Turkey are set up and operated by DMI and DSI. There are two meteorological stations in Karadere and three meteorological stations in Solaklı watersheds. All of the stations in both basins are operated by DMI. These stations measure daily rainfall and temperature. The ones in Karadere Watershed are Dağbaşı DMI and Kayaiçi DMI which are both closed among which Kayaiçi DMI does not have any records. In Solaklı Watershed, there are Çaykara DMI, Köknar DMI and Uzungöl DMI among which only Uzungöl DMI is under operation. Also Köknar DMI has no records. Only Uzungöl DMI, Dağbaşı DMI and Çaykara DMI stations are used for the analysis and processing of meteorological data. Trabzon DMI, Trabzon Meydan DMI and Rize DMI are the stations that are outside the study area. Trabzon DMI was closed in 2005 and then Trabzon Meydan DMI was put into operation.

Mean annual rainfall values and seasonal rainfall values for the spring month of Uzungöl DMI, Dağbaşı DMI and Çaykara DMI are transferred to the median

elevation of the basins in order to acquire a representative mean areal rainfall value for the related watersheds. Mean daily temperature values of the same stations used for rainfall analysis are also used to get a significant relationship with the daily snow covered area data of each basin which are explained later in Chapter 3.2.4.

The networks in two watersheds are insufficient in order to have an idea about the areal mean precipitation values. Only in three stations there are some records of precipitation and the network density is about 500km²/station. According to World Meteorological Organization (WMO), the mountainous regions of temperate zones should have at least 200 km²/station (Usul, 2001). Also the observation periods of the related stations do not seem to be sufficient since Dağbaşı DMI and Çaykara DMI have relatively shorter measured periods and both have discontinuous measurements as well as Uzungöl DMI. The locations of the meteorological stations within the watersheds of Solaklı and Karadere are seen in Figure 3.2. In Table 3.1, there are some properties of the stations which are within the basin. In this table, the coordinate information of Uzungöl DMI and the other big climate stations are taken from the website of DMI. The coordinate information of Çaykara DMI and Dağbaşı DMI are gathered from Meteorological Bulletin of DMI published in 1995.

Table 3.1 Some Properties of the Meteorological Stations within Solaklı and Karadere Watersheds

			Coord	dinates			Mean	
Station No	Station Name	Station Type	X (m)	Y (m)	Elevation (m)	Mean Annual Temperature (°C)	Annual Rainfall (mm)	Observation Period
1962	Uzungöl DMI	Small Climate	608526	4497033	1355.0	8.4	1116.85	1983-2009
1801	Çaykara DMI	Small Climate	611181	4511840	400.0	12.4	998.51	1989-1998
1787	Dağbaşı DMI	Small Climate	577432	4509523	545.0	12.4	646.38	1989-1998
17037	Trabzon DMI	Big Climate	564004	4538814	30.0	14.6	833.94	1975-2005
17038	Trabzon Meydan DMI	Big Climate	564004	4538814	38.8	14.0	000.94	2005-2008
17040	Rize DMI	Big Climate	626164	4544172	8.6	14.1	2246.32	1975-2008

The discharge measurements in the streams in Turkey are made by EIEI and DSI. There are also some separate individual flow gauging stations within the project area, operated by private sector institutions. Average daily discharge measurements are collected from the stations. There are four streamflow gauging stations in Solaklı Watershed and six in Karadere Watershed. 2202, 2240 and 2234, operated by EIEI, 22-44, operated by DSI and 22-222 and 22-208 operated by private sector are the stations that are present in Karadere Watershed. 2203, operated by EIEI, 22-52, 22-57 and 22-07, operated by EIEI, are the streamflow gauging stations that are present in Solaklı Watershed. 2240 namely Karadere-Pervane Köprü streamflow gauging station was in

operation just for two years between the years 1965-1966 and 2203 namely Of Deresi-Dernekpazar streamflow gauging station was in operation between 1943 and 1949 and then it was closed. So these two stations stated above are not used because their observation period is considered to be very short. In Table 3.2 some properties of the stations are seen. Also in Figure 3.2, the network of the streamflow gauging stations is demonstrated.

Table 3.2 Some Characteristics of the Streamflow Gauging Stations within Solaklı and Karadere Watersheds

																E	valu	ated	Yea	rs																							Τ	Ī
Flow Gauging Stations	Elevation (m)	Catchment Area (km²)	o o o i a i i a i o o	Coordinates (Y) (m)	1943	1944 1945	1946 1947	1948 1949	1950	1952	1953	1955 1956	1957	1959	1960 1961	1962 1963	1964 1965	1966	1968	1970	1972	1973 1974	1975	1977	1979	1980	1982	1984	1985 1986	1987	1989	1990	1992 1993	1994	1996	1997 1998	1999	2001	2002	2004	2005	2007	2008	2010
2202					TT														П												П				П								T	Т
Karadere - Ağnas	105	633.2	584351	4520932																																								
22-44																																			П								П	Г
Karadere - Aytaş	527	426.8	576137	4503949																																								
2234																			П												$\mathbf{I}$												T	Т
Karadere - Erikli	1362	204.3	580574	4495249																															П								I	Τ
22-52																																			П								П	Г
Solaklı Stream- Ulucami	295	560.1	605175	4511794																																								
22-57																																											$\Box$	Г
Ögene Stream - Alçakköprü	675	240.1	602665	4502811																																								
22-007																																			П								П	Г
Haldizen Stream - Serah	1114	149.2	609610	4497257																																								L
22-208																																												
Ortacag	870	38.7	579795	4508077																																								
22-222																																											ı	
Canak-II	1880	9.2	583751	4504789																																								

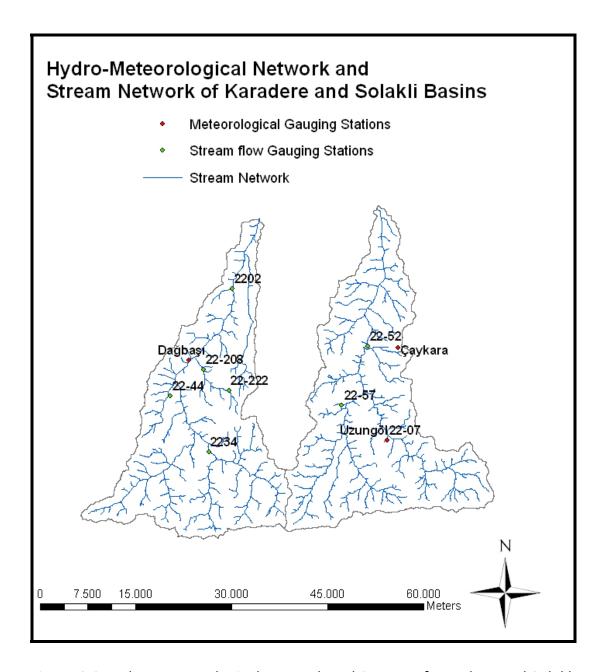


Figure 3.2 Hydro-Meteorological Network and Stream of Karadere and Solaklı

Basins

## 3.2.3 Topographic Data

Topographic map is important to obtain the parameters like linear measures, relief or slope parameters, shape and morphological parameters which influence the surface runoff. The digital elevation models (DEM) of the Karadere and Solaklı basins are shown in Figure 3.3 and Figure 3.4 respectively. These models are acquired from ASTER DEM products which are in 30x30m resolution obtained from internet (Web 3). In Figure 3.3 and Figure 3.4 the outlet points of the basins are the most downstream points that are necessary for this study, in other words the last gauging station or facility site, since more downstream of these points are not used in the modeling studies.

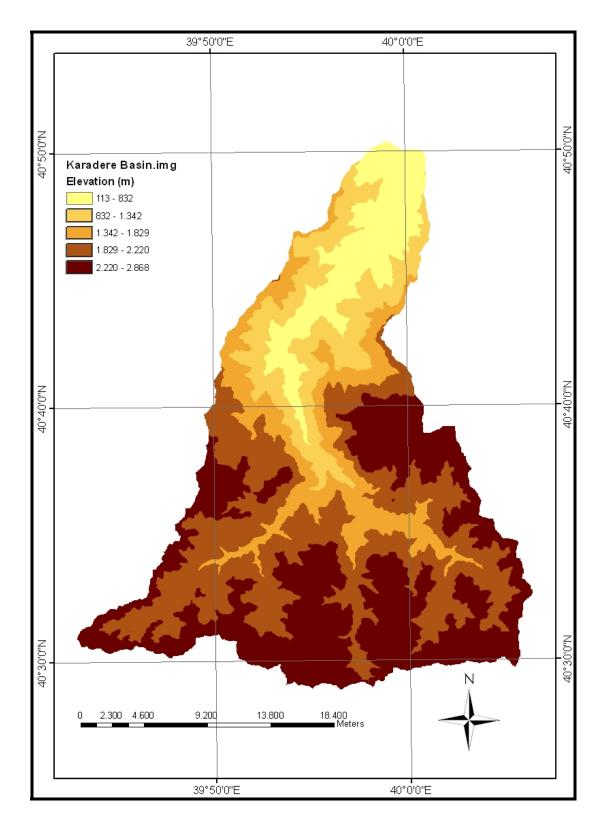


Figure 3.3 DEM of Karadere Basin

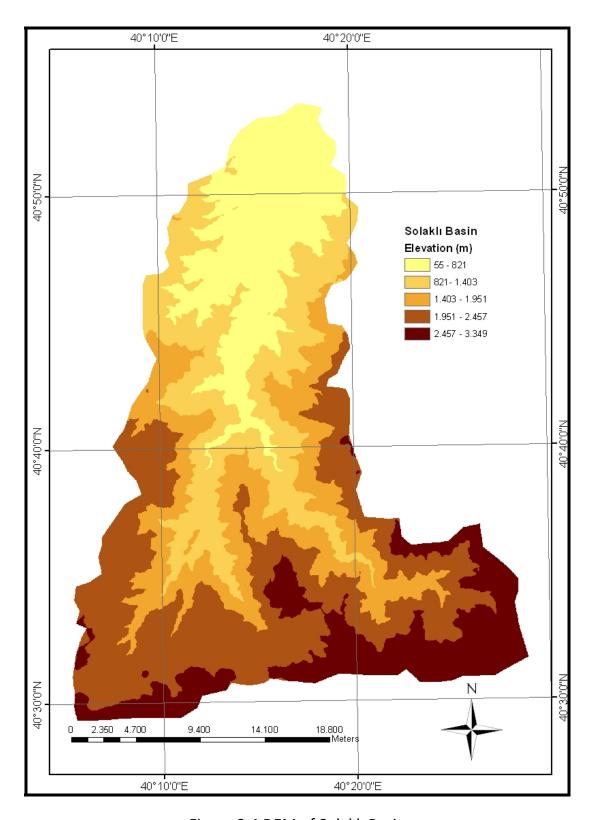


Figure 3.4 DEM of Solaklı Basin

#### 3.2.4 Snow Covered Area Data

In the regions where the altitude is relatively high, snow is the main source of streamflow. According to Usul, 2001; especially in the mountainous regions of Eastern Anatolia, 70% of river flow is due to snowmelt and the snowmelt season in Turkey generally begins in the spring and lasts till early summer season which is a long season. Being a relatively mountainous region, the snowmelt in Eastern Black Sea Basin contributes to the streamflow significantly. Therefore, investigating the effect of snowmelt on streamflow generation, especially in spring months, is important in this study.

In this context; monitoring, modeling and quantification of the snow covered area data (SCA) are critical. In recent years there have been a lot of technological and scientific developments on mapping snow covered areas using remote sensing techniques. MSG SEVIRI is one of the recent satellites that is powerful in mapping snow covered areas (Surer, 2008).

The snow covered area data are gathered from the SEVIRI SR (Snow Recognition) products of the months between January and May of the years 2008 and 2009. A pixel value based algorithm is developed for SR over mountainous areas of Europe. This method is using the satellite images acquired every 15 minutes from a geostationary satellite; Meteosat Second Generations (MSG) instrument Spinning Enhanced Visible and Infra-Red Imager (SEVIRI). Cloud can be distinguished from snow by the algorithms used to produce the SEVIRI SR products (Surer, 2008).

In Figure 3.5, a picture of a SEVIRI SR product is seen. In this figure, snow is shown in white color, clouds are shown in cyan color, water is shown in blue color and land is shown in green color.

The SEVIRI SR products are used indirectly in the seasonal model. With the help of GIS, snow covered area values are represented as a percentage of snow cover to the total area of the related basin. After determining the snow covered areas as percentage, the relationship between mean daily temperature of each basin for the snowmelt season and snow covered area of each basin is studied since mean daily temperature is found to be significant in previous studies (Zaherpour et al., in press).

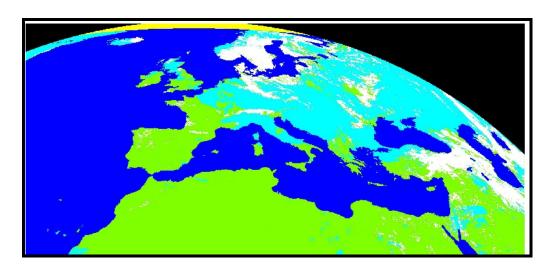


Figure 3.5 An image file from a SEVIRI SR product

## 3.2.5 Characteristics of the Facility Sites

The Eastern Black Sea Basin is the basin with the largest potential for small HEPPs (Yuksel et al, 2008). Solaklı and Karadere basins are the two basins which are very popular in this case. To increase the sample size in the study area, the basins of each project are also delineated besides flow gauging stations.

There are several HEPP projects in both Solaklı and Karadere basins. For Karadere basin, four projects are selected and seven projects are selected for Solaklı watershed. All of these projects are the projects of private companies. Some important properties of these projects were gathered from DSI IV. Planning Directorate. The lists of the projects and their related properties are given below in Table 3.3 and Table 3.4. In Figures 3.6 and 3.7, the locations of the diversion weirs and their related HEPPs can be seen.

Table 3.3 Some Properties of the HEPPs and Diversion Weirs Located within Karadere Watershed

NAME OF THE FACILITY	COORDINATES OF THE DIVERSION WEIR (X, Y) (m) (UTM)	INSTALLED CAPACITY	PROJECT DISCHARGE (m³/s)	PERCENTAGE CORRESPONDING TO PROJECT DISCHARGE (%)	PERIOD OF DISCHARGE MEASUREMENT	DRAINAGE AREA OF THE DIVERSION WEIR, km <sup>2</sup>
Çanak-l REG	4503635, 579975	10.0	1.4	13	1967-2003	5.60
Bangal REG	4495540, 575090	17.0	5.8	13	1967-2003	135.50
Erikli REG	4495600, 580500					209.00
Akkocak REG-HEPP	4496300, 577425	78.0	12.3	13	1967-2001	349.00

Table 3.4 Some Properties of the HEPPs and Diversion Weirs Located within Solaklı Watershed

NAME OF THE FACILITY	COORDINATES OF THE DIVERSION WEIR (X, Y) (m) (UTM)	INSTALLED CAPACITY	PROJECT DISCHARGE (m³/s)	PERCENTAGE CORRESPONDING TO PROJECT DISCHARGE (%)	PERIOD OF DISCHARGE MEASUREMENT	DRAINAGE AREA OF THE DIVERSION WEIR, km <sup>2</sup>
Uzungöl-l HEPP	4498674, 608432	28.2	12.0	8.5	1966-2005	170.80
Arca HEPP	4526774, 607931	16.4	39.0	12.5	1979-2003	734.60
Güneşli-II HEPP	4518660, 606340	12.6	31.0	16.0	1971-2003	653.00
Çaykara HEPP	4512338, 605451	27.0	23.8	18.0	1979-2004	568.00
Ballıca HEPP	4522577, 608148	13.8	39.0	12.0	1979-2006	703.20
Irmak REG (Esentepe HEPP)	4496639, 602194	16.2	10.0	12.0	1968-2003	119.18
Oğlaklı REG (Esentepe HEPP)	4497200, 601208	10.2	10.0	12.0	1900-2003	88.25

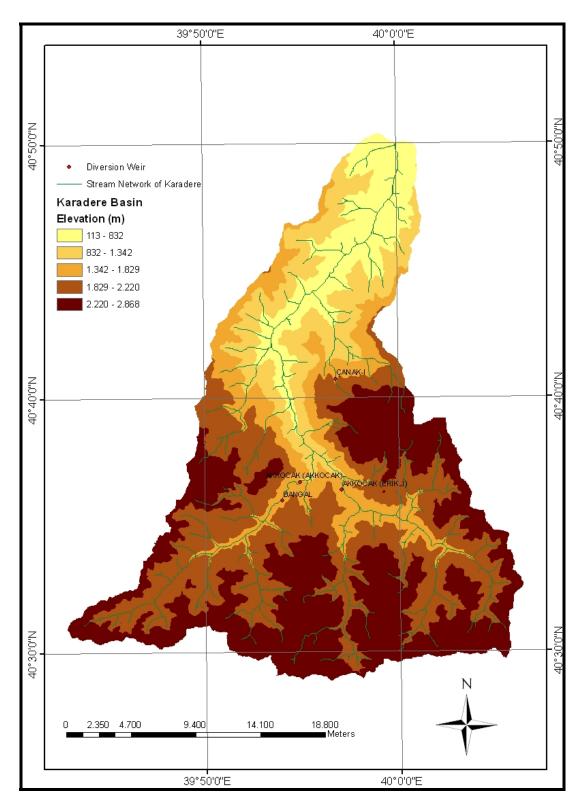


Figure 3.6 The Locations of the Planned Projects within Karadere Basin

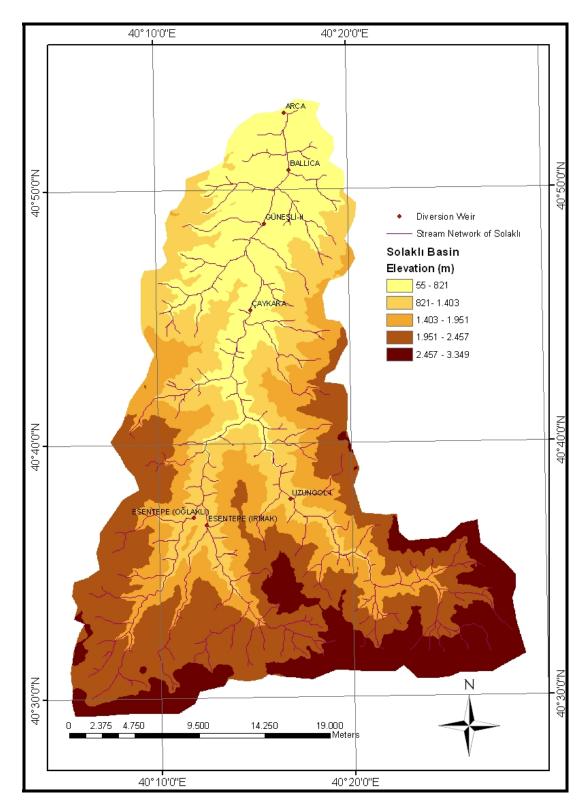


Figure 3.7 The Locations of the Planned Projects within Solaklı Basin

#### **CHAPTER 4**

#### PROCESSING AND ANALYSIS OF DATA

#### 4.1 Rainfall Data

In this study, rainfall data are collected from meteorological stations which are point observations. As stated in Chapter 3.2.2, Dağbaşı DMI is the only meteorological station within Karadere Watershed; Çaykara DMI and Uzungöl DMI are the two stations in Solaklı Watershed that are used as valid rainfall measuring stations. Outside the study area, there are Trabzon DMI and Rize DMI stations. The necessary information about the stations like the periods of observation, coordinates and elevations are given in Table 3.1.

Firstly, the rainfall values of each meteorological station are analyzed. It is seen that there are some missing values and discontinuities in the stations. There are several methods to estimate missing records of a station using the surrounding stations like station average method, normal ratio method, inverse distance weighting method and regression (Dingman, 2002). In this study, the regression method is used since the other methods are not appropriate to be used. The other methods could be used whenever the number of stations is more than one, but for this study only Uzungöl DMI is the appropriate meteorological station that is used for regression analyses since Trabzon Meydan (Trabzon) DMI and Rize DMI are not found to be

suitable to be used to complete the missing values because of low determination coefficient value which is explained in the following paragraph.

Trabzon Meydan DMI is accepted to be the continuation of the closed station Trabzon DMI because their locations are very close. This assumption is accepted also by DMI. So Trabzon Meydan (Trabzon) DMI is the true denotation for these stations.

It is obvious that there is no relationship between the values of Trabzon Meydan (Trabzon) DMI and the values of other stations as the highest value of coefficient of determination (R<sup>2</sup>) is 0.45 which is between Trabzon Meydan (Trabzon) DMI and Çaykara DMI. Besides, the same situation as Trabzon Meydan (Trabzon) DMI is valid for Rize DMI since the highest R<sup>2</sup> value is 0.22 which is not sufficient. Therefore, Trabzon Meydan (Trabzon) DMI and Rize DMI are not used to complete the missing values. The reasons behind the low values of coefficient of determination values of the relationships stated above, the distance and the elevation difference between the meteorological stations Trabzon Meydan (Trabzon) DMI, Rize DMI and the stations inside the selected study area are very big.

November month of 1996 in Uzungöl DMI is missing and since there are no other stations for Uzungöl DMI to complete; the mean monthly rainfall value of November month, 119.36 mm, is accepted to be the missing value of 1996 November of Uzungöl DMI. Also the period for the analyses of rainfall data of Uzungöl DMI is accepted to be between 1991 and 2009 although the

measurements started in 1983. The reason behind this is the discontinuity of Uzungöl DMI rainfall data before the year 1991.

## 4.1.1 Dağbaşı DMI

Dağbaşı DMI started to measure rainfall depths in 1989 and it was closed ain1998. There are discontinuities in the measurements of Dağbaşı DMI. There are not any measured data in 1992 and 1996 whereas some months of 1997 and 1998 are lacking. Furthermore, the data in 1989 are not taken into account because in that year there are not any data in Uzungöl DMI. Three kinds of regression analyses are performed by considering different periods of the year to complete the missing values of Dağbaşı DMI. The independent variable in this model is Uzungöl DMI.

First regression analysis is performed by taking account of whole year data. The second analysis is performed by dividing the year into two halves whereas half years are fall-winter (starting from the beginning of September till the end of February) and spring-summer (starting from the beginning of March till the end of August) periods. The last one is considering each season as winter, spring, summer and fall according to calendar year. The list of the equations and coefficient of determination values are given in Table 4.1.

Table 4.1 The List of Equations and R<sup>2</sup> Values Depending on Different Regression Analyses Between Dağbaşı DMI – Uzungöl DMI

	Fall	Winter	Spring	Summer
Annual		y = 0.523x + 3.9	971, $R^2 = 0.62$ .	
Half- Annual	y = 0,434x + 1	$0,473, R^2 = 0,58$	y = 0,682x - 7,	299, R <sup>2</sup> = 0,71
Seasonal	y = 0.499x + 8.558 $R^2 = 0.71$	y = 0.333x + 14.777 $R^2 = 0.45$	y = 0.740x - 15.656 $R^2 = 0.67$	y = 0.749x - 9.611 $R^2 = 0.55$

The mean annual rainfall values of Dağbaşı DMI were calculated considering the regression equations shown above. The results with the relative errors are given in Table 4.2.

Table 4.2 Mean Annual Rainfall Values, Seasonal Variations and Relative Errors of Dağbaşı DMI for Various Regression Analyses, mm

		Fall	Winter	Spring	Summer
	By Equation		6:	38	
Annual	Observed		6:	34	
	Relative Error (%)			1	
Half-	By Equation	319	)	320	)
Annual	Observed	311		323	3
Ailliuai	Relative Error (%)	3		1	
	By Equation	187	133	164	157
Seasonal	Observed	182	129	173	150
	Relative Error (%)	3	3	5	5

When both the regression equations with determination coefficients and the relative errors are examined; it is seen that the variation of  $R^2$  values and relative errors between seasons are not low and it is decided to complete the missing values of Dağbaşı DMI by annual regression equation y = 0.523x + 3.971,  $R^2 = 0.62$ . The mean monthly rainfall values of Dağbaşı DMI are given below in Table 4.3. Please note that bold values are the values that are completed from Uzungöl DMI by the related equation provided above. The scatter diagram with the accepted regression equation and  $R^2$  value is also given in Figure 4.1.

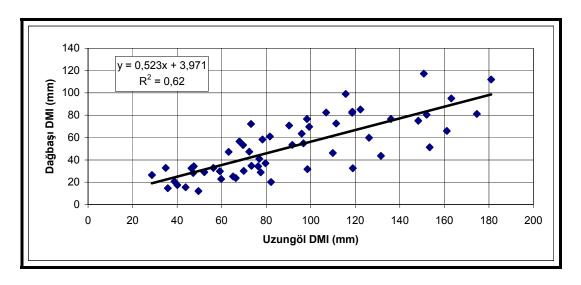


Figure 4.1 Regression Equation of Uzungöl DMI and Dağbaşı DMI

Table 4.3 Mean Monthly Rainfall Values of Dağbaşı DMI, mm

							Months						
Years	January	February	March	April	May	June	July	August	September	October	November	December	Yearly Total
1990	46,2	47,1	33,3	114,1	80,1	76,1	18,3	11,2	62,9	90,1	70,8	34,2	684,4
1991	34,7	41,0	54,9	28,8	95,1	82,5	15,5	53,4	17,5	72,7	34,2	31,7	562,0
1992	84,0	105,7	37,7	45,4	60,1	59,7	70,5	30,2	69,6	59,3	107,2	35,6	765,0
1993	81,1	53,3	30,1	99,0	47,3	76,6	25,2	22,9	32,8	20,6	112,0	32,9	633,8
1994	28,3	85,2	47,2	29,0	20,2	75,1	37,0	12,1	32,5	63,5	83,1	59,8	573,0
1995	23,7	14,7	58,3	82,1	46,2	117,2	72,2	56,6	69,6	80,4	65,9	29,8	716,7
1996	17,7	31,5	22,5	67,8	25,1	52,1	42,7	64,5	61,1	102,3	66,4	45,7	599,4
1997	60,7	53,2	73,2	46,5	54,9	47,0	57,5	31,6	59,2	76,6	26,4	61,0	647,7
1998	43,7	32,5	51,3	53,1	73,6	52,7	31,2	49,7	38,0	40,9	47,8	47,5	562,0
Average	46,7	51,6	45,4	62,9	55,8	71,0	41,1	36,9	49,2	67,4	68,2	42,0	638,2

The mean annual rainfall depth is 638.2 mm for Dağbaşı DMI.

## 4.1.2 Çaykara DMI

Çaykara DMI started to measure rainfall depths in 1989 as the same as Dağbaşı DMI. There are also missing data for this station where in 1996 has no ever data; in 1997 and 1998 there are missing data in some months. The period for the analyses starts also at 1990. The same methodology that is explained in Chapter 4.1.1.1 for completion of missing values of Dağbaşı DMI is applied for Çaykara DMI. The list of the equations and related R<sup>2</sup> values are given in Table 4.4.

Table 4.4 The List of Equations and R<sup>2</sup> Values Depending on Different Regression Analyses between Çaykara DMI – Uzungöl DMI

	Fall	Winter	Spring	Summer
Annual		y = 0.804x + 6	$582, R^2 = 0.65$	
Half- Annual	y = 0,779x + 1	$3,736, R^2 = 0,74$	y = 0,810x + 1,	132, $R^2$ = 0,53
Seasonal	y = 0.901x + 6.646, $R^2 = 0.84$	y = 0.628x + 23.626, $R^2 = 0.64$	y = 1,054x - 36,840, $R^2 = 0,77$	y = 1,062x - 3,342, $R^2 = 0,63$

The mean annual rainfall values of Çaykara DMI are calculated considering the regression equations shown above. The results with the relative errors are given in Table 4.5.

Table 4.5 Mean Annual Rainfall Values and Relative Errors of Çaykara DMI for Various Regression Analyses

		Fall	Winter	Spring	Summer
	By Equation		99	94	
Annual	Observed		10	14	
	Relative Error (%)		2	2	
Half-	By Equation	5	52	44	40
Annual	Observed	50	37	44	47
Alliluai	Relative Error (%)	;	3	2	2
	By Equation	312	244	200	245
Seasonal	Observed	245	322	206	241
	Relative Error (%)	27	24	3	2

If the determination coefficients and relative errors are compared between three different periods, the error variation and the values of  $R^2$  between the seasons can easily be seen. This variation is much higher than that of Dağbaşı DMI. Therefore, the annual regression equation y = 0.804x + 6.582,  $R^2 = 0.65$  is accepted for completion of missing values of Çaykara DMI. The scatter diagram and the accepted regression equation with its  $R^2$  value are given in Figure 4.2. Also the mean monthly rainfall values of Çaykara DMI are given in Table 4.6. Bold values are the values that are completed from Uzungöl DMI.

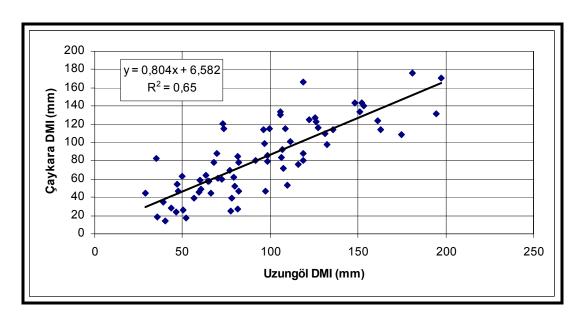


Figure 4.2 Regression Equation of Uzungöl DMI and Çaykara DMI

Table 4.6 Mean Monthly Rainfall Values of Çaykara DMI, mm

Vooro							Months						
Years	January	February	March	April	May	June	July	August	September	October	November	December	Yearly Total
1990	88,6	66,4	39,2	158,6	112,6	114,4	32,8	48,0	86,4	161,8	114,6	77,4	1100,8
1991	115,2	69,4	99,4	24,8	114,6	92,8	28,0	80,4	14,4	101,0	46,2	86,2	872,4
1992	140,6	131,2	57,4	62,2	71,8	83,8	116,2	26,4	127,1	130,4	170,5	48,6	1166,2
1993	109,1	88,3	60,8	76,2	59,5	114,0	57,9	58,9	82,2	34,6	175,7	39,2	956,4
1994	54,4	124,9	64,2	17,0	46,7	144,0	51,8	62,7	23,8	113,8	166,5	122,6	992,4
1995	44,5	18,5	39,4	80,7	52,9	134,0	121,1	78,1	115,3	143,0	124,1	46,1	997,7
1996	27,6	49,0	35,0	104,8	39,1	80,6	66,2	99,6	94,4	157,8	102,5	70,7	927,3
1997	115,4	82,2	97,9	26,9	46,3	78,7	88,9	49,1	134,2	79,8	44,8	84,8	929,1
1998	110,2	87,6	129,9	82,1	113,6	81,4	48,5	76,9	58,8	63,3	74,0	73,6	999,9
Average	89,5	79,7	69,3	70,4	73,0	102,6	67,9	64,5	81,8	109,5	113,2	72,1	993,6

The mean annual rainfall depth is 993.6 mm for Çaykara DMI.

# 4.1.3 Uzungöl DMI

The mean monthly rainfall values of Uzungöl DMI are also given in Table 4.7. The bold value that is the November month of 1996 is completed by accepting the mean November rainfall depth as for 1996 November month as explained before.

Table 4.7 Mean Monthly Rainfall Values of Uzungöl DMI, mm

Vaara							Months						
Years	January	February	March	April	May	June	July	August	September	October	November	December	Yearly Total
1991	73,4	76,9	96,7	77,5	163,1	106,9	43,8	91,7	40,1	111,4	47,5	98,5	1027,5
1992	153,1	194,5	64,4	79,3	107,3	106,5	127,2	50,2	125,5	105,7	197,4	60,5	1371,6
1993	174,6	69,6	69,9	115,7	72,4	136,0	65,1	59,8	34,9	38,8	181,0	56,3	1074,1
1994	47,2	122,3	63,1	52,2	82,2	148,3	79,7	49,6	46,4	95,9	118,7	126,2	1031,8
1995	66,3	35,8	78,3	118,6	109,9	150,8	73,2	68,0	99,3	152,1	161,1	59,2	1172,6
1996	26,2	52,7	35,4	122,1	40,4	92,1	74,1	115,7	109,2	188,1	119,4	79,7	1055,1
1997	108,4	94,1	132,4	81,3	97,3	82,2	102,4	52,9	105,6	98,3	28,7	81,7	1065,3
1998	131,5	118,9	153,4	93,9	133,1	93,1	52,1	87,5	65,0	70,6	83,8	83,3	1166,2
1999	31,9	59,5	108,3	132,9	163,6	82,8	68,2	60,7	72,7	96,7	92,1	34,3	1003,7
2000	171,9	107,2	111,4	71,2	69,8	116,7	38,1	120,3	89,2	154,3	12,2	126,5	1188,8
2001	14,3	87,8	90,0	134,2	154,8	108,6	74,4	69,6	35,6	107,7	154,3	103,7	1135,0
2002	125,8	42,5	100,9	117,6	60,6	152,9	69,2	85,9	82,7	88,5	96,6	116,4	1139,6
2003	50,3	86,9	117,2	88,4	38,5	54,4	61,0	53,4	103,9	155,9	111,4	62,5	983,8
2004	85,5	171,1	129,6	111,6	172,1	139,2	43,2	71,0	39,1	66,3	218,0	71,4	1318,1
2005	84,1	67,5	115,0	120,3	102,7	142,0	30,5	91,7	56,4	229,8	146,2	67,7	1253,9
2006	88,7	86,2	104,8	139,5	118,9	57,5	134,4	15,0	66,0	131,1	185,9	127,9	1255,9
2007	93,1	47,5	119,6	117,7	49,6	62,4	95,8	95,6	38,7	74,9	143,8	87,8	1026,5
2008	123,2	62,0	40,5	75,5	114,2	111,8	76,8	67,6	85,3	105,8	22,8	72,6	958,1
2009	0,0	123,9	140,7	65,6	96,5	79,9	100,9	37,8	122,7	49,6	246,9	76,4	1140,9
Average	86,8	89,8	98,5	100,8	102,5	106,5	74,2	70,7	74,6	111,7	124,6	83,8	1124,7

Mean annual rainfall value for Uzungöl DMI is 1124.7 mm.

#### 4.1.4 Areal Estimation of Rainfall

For many applications in hydrology, areal precipitation values are input to hydrologic models. For the model in this study, mean areal rainfall values that are representative for each basin are determined for the model. There are several techniques used to estimate mean areal rainfall values of a particular region, generally a drainage basin. There are principally two methods: first one is direct weighted averages and the second one is surface fitting methods. The first method includes: arithmetic average, thiessen polygons

and two-axis method. The second one includes isohyetal method, kriging, conventional hypsometric method and algorithmic hypsometric methods. The techniques to be used depend on several factors such as the number of stations, objective of study and the nature of the region (Dingman, 2002).

In this study a different technique which belongs to the direct weighted averages method is used to estimate areal rainfall. In this technique the orographic effect on rainfall is considered. This means that precipitation increases with the elevation. The formula is given by (Web 4):

$$P_{h} = P_{ref} (1 + (\frac{h}{h_{ref}})p_{cor})$$
 (4.1)

where  $P_{ref}$  is the precipitation at the observation station,  $P_h$  is corrected precipitation, h is the height to which the precipitation is corrected,  $h_{ref}$  is the height of observation station and  $p_{cor}$  is the correction factor.  $p_{cor}$  is assumed to be 5% (A.A. Şorman, personal communication, June 25 2010). However this formula is wrong since the precipitation amount at the target point becomes always greater than the precipitation amount in reference station. So this formula is corrected as:

$$P_{h} = P_{ref} \left(1 + \left(\frac{h_{ref} - h}{100}\right) p_{cor}\right)$$
(4.2)

is the modified formula after the precipitation amount in the observation station is corrected by catch deficiency by the formula given:

$$P_{\text{ref}} = P \times p_{\text{catch}} \tag{4.3}$$

Here the  $p_{catch}$  is assumed to be 0.3 %. This correction factor is also gathered by personal communication.

The formula mentioned in Equation 4.2 is used to calculate the rainfall amount at the median elevation of each basin. Median elevation of each basin is assumed to represent mean areal rainfall of the basin.

## 4.2 Temperature Data

Temperature records are also collected from Çaykara DMI, Dağbaşı DMI, Trabzon Meydan (Trabzon) DMI, Rize DMI and Uzungöl DMI. The main purpose to collect temperature records of the related meteorological stations is to determine mean daily temperature values of each basin for certain periods of 2008 and 2009 years to look for a relationship with snow covered areas (SCA) for Solaklı and Karadere basins in order to replace mean annual

or seasonal temperature values instead of SCA values in the final regression model. The station used for the analysis is Uzungöl DMI since Dağbaşı DMI and Çaykara DMI were closed at 1998. In addition, Trabzon Meydan (Trabzon) DMI and Rize DMI are not used in calculations because they locate outside the study areas.

To estimate the missing data of Dağbaşı DMI and Çaykara DMI, seasonal regression analyses are performed and the results are given in Figure 4.3 and 4.4.

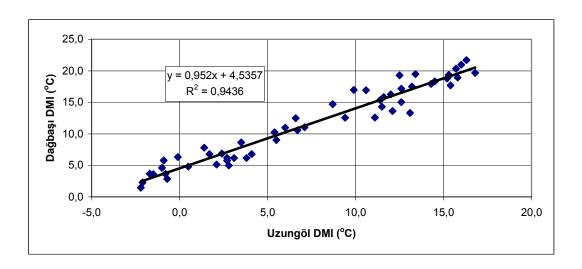


Figure 4.3 Regression Equation of Uzungöl DMI and Dağbaşı DMI

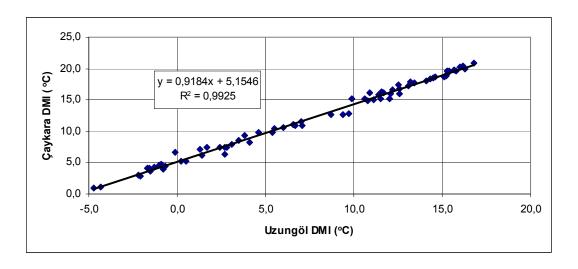


Figure 4.4 Regression Equation of Uzungöl DMI and Çaykara DMI

The mean monthly temperature values of Uzungöl DMI are given below in Table 4.8. Although temperature recording of Uzungöl DMI started in 1983, the beginning year of records of Uzungöl DMI is accepted to be 1991 since the data before are not continuous.

The mean daily temperature values of Uzungöl DMI for the years 2008 and 2009 are available and these data are ready to be used to make a correlation analysis with SCA data of the related basins.

Table 4.8 Mean Monthly Temperature Values of Uzungöl DMI, °C

								Months					
Years	January	February	March	April	May	June	July	August	September	October	November	December	Mean Annual
1991	-1,7	-1,5	2,7	9,4	10,6	14,3	16,8	15,8	12,6	11,6	5,5	0,5	8,1
1992	-4,7	-4,3	1,3	6,6	9,7	14,6	15,1	16,2	11,7	10,9	4,6	-1,3	6,7
1993	-2,2	-2,1	2,4	6,6	11,5	13,2	14,5	16,0	12,5	9,9	1,4	3,8	7,3
1994	3,1	-0,7	2,8	11,1	12,1	13,1	15,4	15,3	16,3	12,6	4,1	-0,8	8,7
1995	2,7	1,7	6,0	7,1	12,0	15,2	15,3	15,7	13,4	8,7	5,4	-0,1	8,6
1996	1,0	2,8	1,8	5,9	14,3	12,5	17,2	16,4	13,5	9,7	5,4	6,3	8,9
1997	0,2	-2,4	-1,6	7,0	12,2	14,1	15,3	15,8	10,8	11,4	6,7	3,5	7,8
1998	-0,9	-1,0	2,1	10,6	12,6	14,7	16,9	17,3	14,5	11,7	8,0	3,7	9,2
1999	2,5	2,8	4,2	7,7	10,2	14,5	17,6	17,3	13,9	10,2	4,8	4,4	9,2
2000	-2,7	-1,1	1,2	11,2	10,1	13,2	17,8	16,5	13,6	9,4	6,3	2,0	8,1
2001	1,6	2,5	7,7	9,4	9,9	14,5	17,7	18,0	15,2	8,9	5,7	3,3	9,5
2002	-3,0	3,1	5,3	6,3	10,8	14,2	18,0	16,3	15,4	11,8	7,9	-1,7	8,7
2003	3,1	-1,5	-0,9	6,4	13,1	13,3	16,0	16,2	13,0	11,2	4,8	2,7	8,1
2004	2,6	0,3	3,4	7,1	10,3	13,5	15,5	16,8	13,7	11,5	5,7	0,2	8,4
2005	1,1	1,0	2,1	8,5	11,7	12,9	17,4	17,7	13,9	8,0	5,3	3,8	8,6
2006	-2,0	2,1	5,2	8,2	11,0	15,5	15,0	19,7	14,0	11,2	4,2	-2,1	8,5
2007	0,2	0,2	2,7	3,6	15,7	15,6	17,1	18,1	15,0	12,3	4,9	0,3	8,8
2008	-4,5	-2,8	7,8	10,3	9,2	13,6	16,4	17,6	14,4	10,3	7,1	1,3	8,4
2009	2,2	4,1	3,5	5,8	10,4	15,6	16,8	14,3	13,0	12,7	5,5	5,5	9,1
Average	-0,1	0,2	3,1	7,8	11,4	14,1	16,4	16,7	13,7	10,7	5,4	1,9	8,5

# **4.2.1** Areal Estimation of Temperature

Areal seasonal estimation of mean temperature is done after finding a relationship between snow covered area (%) of the basins and mean daily temperature values of Uzungöl DMI.

In this study, areal temperature amount of each basin is determined by the saturated adiabatic lapse rate technique. In this technique, the saturated air temperature decreases 0.5°C every 100m. It means that the lapse rate is 0.5°C/100m. Here, the mean areal temperature of the basin is calculated as transferring the mean seasonal (spring) temperature of Uzungöl DMI to the median basin elevations of each basin. Since mean areal temperature values are only used for seasonal (spring) models, the mean seasonal temperature values are considered. Also, the temperature values at median elevations of each basin are assumed to represent areal temperature value of each basin. Median basin elevation is determined by deriving the hypsometric curve of each basin and extracting the elevation corresponding to 50% of cumulative area. The equation to estimate the areal temperature value of the basin is given by:

$$T_{h} = T_{ref} + (\frac{(href - h)}{100}) \times 0.5$$
 (4.4)

where  $T_{ref}$  is the mean seasonal temperature value for the observation station,  $T_h$  is mean seasonal temperature value of the basin, h is the height to which the temperature is transferred,  $h_{ref}$  is the height of observation station.

### 4.3 Discharge Data

There are five stream flow gauging stations in Karadere Basin and three in Solaklı Basin as stated in Chapter 3.2.2. Discharge records of the stations are carefully observed and analyzed and some corrections are made. In this chapter, the analyses of discharge data are given in two parts. In the first

part; the stream flow gauging stations in Karadere Basin and in the second part the ones in Solaklı Basin are analyzed.

For the analyses of discharge data of stream flow gauging stations, a macro program called "Su Temini", developed by Beray Engineering, is used. The correlation and regression studies between the stations, calculating mean monthly and yearly discharge data and deriving the flow duration curves for the related basins are performed by this macro. This macro is one of the most practical and useful tools in deriving flow duration curves of the basins which are very important in this study.

#### 4.3.1 Karadere Basin

2202 Ağnas stream flow gauging station is the station in the most downstream. 22-44 Aytaş and 2234 Erikli are the stations that are in the upstream part of the basin. Besides these; 22-208 and 22-222 are the stations operated by private companies that are present on the tributaries of Karadere.

Firstly, the stream flow periods of the stations used to obtain flow duration curves are determined.

In Karadere Basin; for 2202 Ağnas; the period between 1967 and 2009 is selected since 43 years of measured discharge data seem to be sufficient and the years before 1967 are not continuous (See Table 3.2). Measurements for

22-44 Aytaş started in 1978 and there are measurements till 2009. Measurements in 22-44 Aytaş are not continuous but they are completed by regression equation with 2202 Ağnas being the independent variable. Also for 2234 Erikli, the measurements start in 1965 and ended in 1974. The stream flow period of 2234 Erikli is extended till 2009 by completing the missing values from 2202 Ağnas. Also the stream flow period this station is accepted to start in 1967 to be consistent with the beginning year of other stations in Karadere Basin.

When the discharge records in both 22-44 Aytaş and 2202 Ağnas are observed; it is seen that some of the records of 22-44 Aytaş is higher than that of 2202 Ağnas. This is physically impossible since 2202 Ağnas locates more downstream of 22-44 Aytaş (See Figure 3.2). This issue may be because of some measurement errors in one of the stations. To correct these errors; 2202 Ağnas which is the more downstream station is accepted to be more reliable and the discharge values of 22-44 Aytaş which are greater than 2202 Ağnas are corrected by carrying the discharge values of 2202 Ağnas of the day that the discharge values of 22-44 are greater, to 22-44 Aytaş by the drainage are ratio method which is given below. There are not any values in 2234 Erikli that are greater than 2202 Ağnas.

$$Q_{22-44Aytas} = Q_{2202Agnas} \left( \frac{A_{22-44Aytas}}{A_{2202Agnas}} \right)$$
(4.5)

Secondly, the missing data of 22-44 Aytaş is completed by using the values of 2202 Ağnas via regression equation. The equation is given below in Figure 4.5.

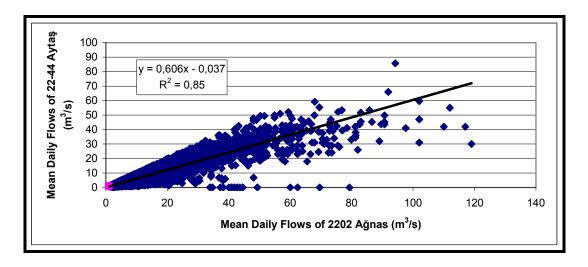


Figure 4.5 Regression Equation of 2202 Ağnas and 22-44 Aytaş

The regression equation between 2202 Ağnas and 2234 Erikli is also given below in Figure 4.6. The missing values of 2234 Erikli between the years 1968 and 2009 are completed from 2202 Ağnas.

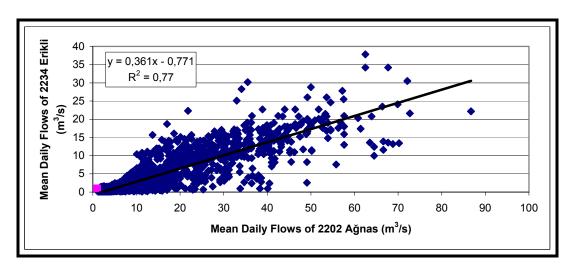


Figure 4.6 Regression Equation of 2202 Ağnas and 2234 Erikli

The mean monthly discharge values of 2202 Ağnas, 22-44 Aytaş, 2234 Erikli, 22-208 and 22-222 are given below in Table 4.9, 4.10, 4.11, 4.12 and 4.13 respectively. Please note that in Table 4.10 and 4.11 the bold values in are the values that are completed from 2202 Ağnas by using the daily flows. The mean monthly values of 2202 Ağnas, 22-44 Aytaş, 2234 Erikli, 22-208 and 22-222 for each year are demonstrated in Figure 4.7, 4.8 and 4.9, 4.10 and 4.11 respectively. The mean annual flows of 2202 Ağnas, 22-44 Aytaş, 2234 Erikli, 22-208 and 22-222 for each year are given in Figure 4.12, 4.13, 4.14, 4.15 and 4.16 respectively.

Table 4.9 Mean Monthly Discharge Values of 2202 Ağnas, m³/s

1968       4.56       4.43       7.80       5.18       5.72       9.05       42.35       40.48       18.15       6.00       4.93         1969       7.42       7.55       3.61       2.24       3.70       7.78       22.31       30.04       7.64       4.42       2.87         1970       6.43       5.04       3.85       3.96       5.40       12.51       26.10       22.85       8.18       2.77       6.69         1971       12.77       7.99       6.73       3.59       3.17       10.88       19.95       31.95       20.39       7.37       4.27         1972       7.90       5.89       5.70       2.42       3.41       6.12       41.15       35.68       22.75       7.53       4.18         1973       5.73       4.49       2.32       3.73       5.89       7.40       20.51       25.78       21.81       6.69       5.15         1974       3.99       9.87       8.31       4.27       4.85       12.29       22.16       29.50       11.96       4.01       4.30         1975       1.83       1.99       3.52       2.83       4.98       10.81       35.95       30.79	9 4.85	Annual
1968       4.56       4.43       7.80       5.18       5.72       9.05       42.35       40.48       18.15       6.00       4.93         1969       7.42       7.55       3.61       2.24       3.70       7.78       22.31       30.04       7.64       4.42       2.87         1970       6.43       5.04       3.85       3.96       5.40       12.51       26.10       22.85       8.18       2.77       6.69         1971       12.77       7.99       6.73       3.59       3.17       10.88       19.95       31.95       20.39       7.37       4.27         1972       7.90       5.89       5.70       2.42       3.41       6.12       41.15       35.68       22.75       7.53       4.18         1973       5.73       4.49       2.32       3.73       5.89       7.40       20.51       25.78       21.81       6.69       5.15         1974       3.99       9.87       8.31       4.27       4.85       12.29       22.16       29.50       11.96       4.01       4.30         1975       1.83       1.99       3.52       2.83       4.98       10.81       35.95       30.79	4.85	
1969       7.42       7.55       3.61       2.24       3.70       7.78       22.31       30.04       7.64       4.42       2.87         1970       6.43       5.04       3.85       3.96       5.40       12.51       26.10       22.85       8.18       2.77       6.69         1971       12.77       7.99       6.73       3.59       3.17       10.88       19.95       31.95       20.39       7.37       4.27         1972       7.90       5.89       5.70       2.42       3.41       6.12       41.15       35.68       22.75       7.53       4.18         1973       5.73       4.49       2.32       3.73       5.89       7.40       20.51       25.78       21.81       6.69       5.15         1974       3.99       9.87       8.31       4.27       4.85       12.29       22.16       29.50       11.96       4.01       4.30         1975       1.83       1.99       3.52       2.83       4.98       10.81       35.95       30.79       10.23       6.30       3.76         1976       7.67       2.91       4.38       5.85       4.78       9.37       27.00       42.32		12.57
1970       6.43       5.04       3.85       3.96       5.40       12.51       26.10       22.85       8.18       2.77       6.69         1971       12.77       7.99       6.73       3.59       3.17       10.88       19.95       31.95       20.39       7.37       4.27         1972       7.90       5.89       5.70       2.42       3.41       6.12       41.15       35.68       22.75       7.53       4.18         1973       5.73       4.49       2.32       3.73       5.89       7.40       20.51       25.78       21.81       6.69       5.15         1974       3.99       9.87       8.31       4.27       4.85       12.29       22.16       29.50       11.96       4.01       4.30         1975       1.83       1.99       3.52       2.83       4.98       10.81       35.95       30.79       10.23       6.30       3.76         1976       7.67       2.91       4.38       5.85       4.78       9.37       27.00       42.32       16.50       6.38       5.44	3.43	12.67
1971       12.77       7.99       6.73       3.59       3.17       10.88       19.95       31.95       20.39       7.37       4.27         1972       7.90       5.89       5.70       2.42       3.41       6.12       41.15       35.68       22.75       7.53       4.18         1973       5.73       4.49       2.32       3.73       5.89       7.40       20.51       25.78       21.81       6.69       5.15         1974       3.99       9.87       8.31       4.27       4.85       12.29       22.16       29.50       11.96       4.01       4.30         1975       1.83       1.99       3.52       2.83       4.98       10.81       35.95       30.79       10.23       6.30       3.76         1976       7.67       2.91       4.38       5.85       4.78       9.37       27.00       42.32       16.50       6.38       5.44	1.74	8.44
1972       7.90       5.89       5.70       2.42       3.41       6.12       41.15       35.68       22.75       7.53       4.18         1973       5.73       4.49       2.32       3.73       5.89       7.40       20.51       25.78       21.81       6.69       5.15         1974       3.99       9.87       8.31       4.27       4.85       12.29       22.16       29.50       11.96       4.01       4.30         1975       1.83       1.99       3.52       2.83       4.98       10.81       35.95       30.79       10.23       6.30       3.76         1976       7.67       2.91       4.38       5.85       4.78       9.37       27.00       42.32       16.50       6.38       5.44	7.52	9.28
1973     5.73     4.49     2.32     3.73     5.89     7.40     20.51     25.78     21.81     6.69     5.15       1974     3.99     9.87     8.31     4.27     4.85     12.29     22.16     29.50     11.96     4.01     4.30       1975     1.83     1.99     3.52     2.83     4.98     10.81     35.95     30.79     10.23     6.30     3.76       1976     7.67     2.91     4.38     5.85     4.78     9.37     27.00     42.32     16.50     6.38     5.44	5.64	11.22
1974     3.99     9.87     8.31     4.27     4.85     12.29     22.16     29.50     11.96     4.01     4.30       1975     1.83     1.99     3.52     2.83     4.98     10.81     35.95     30.79     10.23     6.30     3.76       1976     7.67     2.91     4.38     5.85     4.78     9.37     27.00     42.32     16.50     6.38     5.44	4.30	12.25
1975     1.83     1.99     3.52     2.83     4.98     10.81     35.95     30.79     10.23     6.30     3.76       1976     7.67     2.91     4.38     5.85     4.78     9.37     27.00     42.32     16.50     6.38     5.44	4.66	9.51
1976 7.67 2.91 4.38 5.85 4.78 9.37 27.00 42.32 16.50 6.38 5.44	5.53	10.09
	2.42	9.62
1977   8.68   4.27   3.44   3.22   4.68   9.39   23.78   34.44   20.18   11.06   5.47	5.44	11.50
	5.55	11.18
	5.39	15.74
	3.76	12.29
	2.74 6.22	10.75
	4.01	10.86
	4.64	10.20
	4.04	11.43
	2.23	9.72
	3.22	10.60
	4.45	13.70
1988 5.88 9.76 6.45 5.97 6.82 9.71 33.14 48.37 27.28 10.32 6.38	4.63	14.56
1989 14.12 14.17 5.95 4.52 6.27 10.43 33.84 17.43 10.47 4.08 2.97	5.55	10.82
1990 7.75 4.84 8.19 4.57 4.41 9.97 41.88 31.22 16.61 5.85 4.71	3.95	12.00
1991 12.03 15.70 3.26 1.60 8.20 10.95 28.68 22.12 10.29 4.16 5.73	3.29	10.50
1992 2.09 6.25 3.01 3.13 7.50 15.46 33.11 31.96 23.79 7.83 4.28	4.31	11.89
1993 7.71 10.11 10.02 6.75 5.16 13.23 32.86 49.65 40.68 6.20 5.18	3.10	15.89
1994 1.92 4.37 5.39 2.88 5.97 10.40 19.76 14.15 6.38 4.30 3.31	1.96	6.73
	6.32	10.01
	5.01	10.68
	6.96	9.41
	2.69	10.03
	7.12	11.60
	5.53	10.28
2001 8.45 5.82 3.31 2.33 3.98 11.77 21.90 21.79 8.92 3.75 2.93 2002 2.58 6.03 4.90 5.54 5.50 10.15 27.35 25.12 16.19 5.50 4.10	1./1 6.67	8.06 9.97
	5.33	8.61
	4.88	14.69
	3.30	11.80
	2.93	14.34
	2.66	12.56
	4.89	10.53
	6.73	11.04
Average 6.53 7.26 5.31 4.23 5.45 10.82 28.82 33.52 16.28 6.74 4.75	4.45	11.18

Table 4.10 Mean Monthly Discharge Values of 22-44 Aytaş, m³/s

Years	Months											Mean	
	10	11	12	1	2	3	4	5	6	7	8	9	Annual
1967	1.58	0.82	0.99	1.76	3.12	5.12	16.57	29.82	11.63	9.39	7.21	2.90	7.58
1968	2.72	2.65	4.69	3.10	3.43	5.45	25.62	24.49	10.96	3.60	2.95	2.04	7.64
1969	4.46	4.54	2.15	1.32	2.20	4.67	13.48	18.16	4.59	2.64	1.70	1.02	5.08
1970	3.86	3.02	2.29	2.36	3.24	7.54	15.77	13.80	4.92	1.64	4.02	4.52	5.58
1971	7.70	4.80	4.04	2.14	1.88	6.55	12.05	19.32	12.31	4.43	2.55	3.38	6.76
1972	4.75	3.53	3.42	1.43	2.03	3.67	24.89	21.58	13.75	4.52	2.50	2.57	7.39
1973	3.44	2.68	1.37	2.22	3.53	4.45	12.39	15.58	13.17	4.02	3.09	2.79	5.73
1974	2.38	5.94	5.00	2.55	2.90	7.41	13.39	17.83	7.21	2.39	2.57	3.32	6.07
1975	1.07	1.17	2.09	1.67	2.98	6.51	21.74	18.62	6.16	3.78	2.24	1.43	5.79
1976	4.61	1.73	2.62	3.51	2.86	5.64	16.32	25.60	9.96	3.83	3.26	3.26	6.93
1977 1978	<b>5.22</b> 4.36	<b>2.55</b> 4.94	<b>2.04</b> 3.43	<b>1.92</b> 2.56	<b>2.80</b> 4.80	<b>5.65</b> 8.88	<b>14.37</b> 20.21	<b>20.83</b> 23.68	<b>12.19</b> 14.65	<b>6.66</b> 5.29	<b>3.28</b> 2.32	<b>3.32</b> 1.66	6.74 8.07
1978	1.85	1.92	2.80	2.98	3.63	7.61	18.33	22.53	11.32	5.65	2.43	0.86	6.83
1979	2.04	6.84	3.11	2.96	2.22	7.01	16.33	18.19	5.33	0.75	1.87	1.85	5.67
1981	2.14	3.17	3.99	2.49	2.46	6.26	15.92	25.68	18.12	6.96	3.28	4.20	7.89
1982	2.75	4.60	2.81	3.20	3.40	5.49	28.11	24.59	8.40	5.26	2.66	2.76	7.84
1983	3.50	3.31	2.37	2.74	3.66	10.69	16.30	24.80	11.64	3.13	2.63	3.26	7.34
1984	5.25	9.81	3.21	1.50	1.55	4.15	15.91	21.44	8.63	4.63	4.17	2.41	6.89
1985	1.92	1.80	1.29	1.55	2.50	4.48	18.80	19.29	11.51	4.31	1.46	1.31	5.85
1986	5.72	4.16	3.84	3.50	4.28	5.50	12.86	17.19	12.07	3.55	1.99	1.91	6.38
1987	5.21	6.07	4.54	5.83	4.79	6.25	18.86	30.85	12.86	5.73	4.07	1.98	8.92
1988	1.59	5.60	3.45	2.57	2.73	4.93	17.77	24.00	18.79	7.11	3.83	2.77	7.93
1989	8.52	8.55	3.57	2.70	3.76	6.28	20.47	10.52	6.30	2.43	1.76	3.32	6.52
1990	4.66	2.89	4.92	2.73	2.63	6.00	25.34	18.88	10.03	3.51	2.82	2.36	7.23
1991	7.25	9.47	1.94	0.93	4.93	6.60	17.34	13.36	6.20	2.48	3.43	1.96	6.32
1992	1.50	4.51	1.76	1.55	1.95	8.21	22.34	23.87	20.42	4.45	2.35	2.91	7.98
1993	4.63	6.09	6.03	4.05	3.09	7.98	19.87	30.04	24.61	3.72	3.10	1.84	9.59
1994	1.12	2.61	3.23	1.71	3.58	6.26	11.93	8.54	3.83	2.57	1.97	1.15	4.04
1995	0.94	1.43	1.50	2.05	1.98	4.12	16.11	19.23	13.48	4.89	2.45	2.15	5.86
1996	5.83	9.94	4.37	2.48	1.95	3.63	12.98	23.20	9.30	2.54	2.78	3.60	6.88
1997	3.87	2.86	3.58	3.47	3.32	4.48	14.97	14.55	6.40	3.54	2.74	4.18	5.66
1998	5.75	<b>4.40</b> 2.31	<b>1.72</b> 5.74	<b>2.60</b> 1.30	<b>4.05</b> 1.35	6.09	<b>14.12</b> 22.84	17.49	<b>8.34</b> 12.21	<b>2.63</b> 4.14	<b>3.68</b> 1.82	<b>1.59</b> 2.85	6.04
1999 2000	1.10 2.76	3.61	2.69	1.60	2.11	4.47 4.82	24.38	29.41 14.71	8.00	1.68	1.82	1.17	7.46 5.72
2000	3.42	3.57	1.30	1.00	0.82	8.25	16.81	17.39	6.91	2.17	1.07	0.81	5.72
2001	0.99	1.68	1.59	1.65	2.73	7.83	20.27	21.51	11.88	3.27	1.68	1.72	6.40
2002	1.92	1.43	1.18	1.48	1.20	1.82	21.69	18.60	5.83	1.42	0.97	1.72	4.90
2003	1.30	3.98	1.66	1.40	2.21	16.08	22.01	29.96	18.34	3.61	1.71	1.54	8.65
2005	1.13	1.08	1.04	1.01	1.48	5.72	31.39	29.10	11.95	3.85	1.89	1.40	7.59
2006	6.17	9.93	5.03	1.70	2.06	7.04	27.28	27.74	6.95	4.06	1.66	1.22	8.40
2007	2.51	8.95	3.03	2.43	2.74	7.64	13.12	36.88	5.95	2.96	1.64	1.24	7.42
2008	1.38	2.63	2.22	1.77	3.01	17.73	20.93	13.73	7.75	2.46	2.49	1.48	6.47
2009	2.07	1.42	1.27	1.34	1.96	7.13	11.29	3.31	12.10	4.87	2.29	2.81	4.32
Average	3.42	4.16	2.90	2.23	2.79	6.57	18.45	20.93	10.63	3.87	2.60	2.28	6.74

Table 4.11 Mean Monthly Discharge Values of 2234 Erikli, m³/s

Years	Months											Mean	
	10	11	12	1	2	3	4	5	6	7	8	9	Annual
1967	0.49	0.36	0.43	0.41	0.57	0.98	3.20	17.84	5.37	3.34	1.48	0.68	2.93
1968	0.72	0.93	1.31	0.93	0.82	1.84	13.97	12.98	5.76	1.69	0.57	0.38	3.49
1969	0.71	1.48	1.10	0.53	0.45	1.45	7.39	12.22	1.89	0.98	0.52	0.34	2.42
1970	0.56	0.40	0.18	0.20	0.48	3.26	9.21	11.77	3.51	0.69	0.51	0.50	2.61
1971	1.26	0.75	1.47	0.52	0.19	1.56	6.67	12.70	7.58	1.34	0.83	0.39	2.94
1972	0.77	0.67	1.01	0.62	0.68	1.83	16.02	13.93	11.87	3.28	0.75	0.62	4.34
1973	0.71	0.69	0.82	0.55	1.10	1.20	8.63	13.48	8.33	1.99	0.57	0.45	3.21
1974	0.48	1.38	0.67	0.59	0.64	2.77	5.86	15.84	5.27	0.92	0.43	0.37	2.94
1975	0.03	0.06	0.50	0.27	1.03	3.13	12.21	10.34	2.92	1.50	0.61	0.19	2.73
1976	2.00	0.28	0.81	1.34	0.96	2.61	8.98	14.51	5.19	1.53	1.19	1.19	3.38
1977	2.36	0.77	0.47	0.39	0.92	2.62	7.81	11.66	6.51	3.22	1.22	1.25	3.27
1978 1979	2.58 0.37	2.16 0.83	1.58 1.51	0.92 1.10	2.22 1.24	4.22 3.11	13.30 12.02	17.71 14.17	9.25	2.41 3.32	1.43 0.91	1.17 0.59	4.91 3.67
1979	2.05	3.55	1.51	1.10	1.46	3.11 4.78	8.64	10.33	4.86 2.46	0.31	0.91	0.59	3.07
1980	0.40	0.94	1.03	0.47	0.35	2.22	6.59	13.01	7.39	2.77	1.08	1.47	3.11
1982	0.40	1.49	0.71	0.73	1.03	1.88	14.55	10.76	3.60	2.08	0.66	0.68	3.23
1983	1.16	0.99	0.50	0.66	1.01	4.61	6.94	10.84	5.76	0.90	0.64	0.90	2.91
1984	2.38	5.10	1.17	0.16	0.21	1.72	8.73	12.03	4.39	2.01	1.73	0.69	3.36
1985	0.47	0.32	0.05	0.24	0.74	1.92	10.45	10.75	6.11	1.82	0.16	0.12	2.76
1986	2.67	1.73	1.54	1.34	1.80	2.53	6.92	9.49	6.45	1.37	0.46	0.44	3.06
1987	1.99	2.71	1.44	2.02	1.80	2.19	9.55	16.58	5.81	2.44	2.73	0.83	4.17
1988	1.43	2.75	1.56	1.38	1.69	2.74	11.19	16.69	9.08	2.95	1.53	0.90	4.49
1989	4.33	4.35	1.38	0.86	1.49	2.99	11.45	5.52	3.01	0.70	0.35	1.30	3.14
1990	2.03	0.97	2.18	0.88	0.82	2.83	14.35	10.50	5.23	1.34	0.93	0.65	3.56
1991	3.57	4.90	0.42	0.01	2.19	3.18	9.58	7.21	2.95	0.73	1.30	0.42	3.04
1992	0.13	1.49	0.33	0.36	1.94	4.81	11.18	10.77	7.82	2.06	0.77	0.79	3.54
1993	2.01	2.88	2.85	1.66	1.09	4.00	11.09	17.15	13.92	1.47	1.10	0.35	4.96
1994	0.04	0.81	1.17	0.28	1.38	2.98	6.36	4.34	1.53	0.78	0.46	0.11	1.69
1995	0.70	0.64	1.32	0.88	0.67	1.95	6.83	9.29	5.74	3.69	1.00	1.51	2.85
1996 1997	4.09	5.22	2.00	0.81	0.59	1.19	5.59	10.46	3.73	0.87	1.40	1.04	3.08 2.63
1997	1.56 2.68	0.96 1.88	1.38 0.28	1.32 0.80	1.23 1.66	1.92 2.88	8.17 7.66	7.92 9.67	3.06 4.22	1.36 0.82	0.89 1.44	1.74 0.21	2.85
1999	0.28	0.96	2.22	0.21	0.43	2.28	9.56	15.47	5.26	1.82	0.71	1.80	3.42
2000	1.25	1.76	1.30	1.04	1.95	3.64	11.42	6.49	4.51	0.54	0.22	1.24	2.95
2001	2.28	1.33	0.43	0.08	0.67	3.48	7.14	7.10	2.45	0.58	0.30	0.01	2.15
2002	0.29	1.41	1.00	1.23	1.22	2.89	9.10	8.30	5.07	1.22	0.71	1.64	2.84
2003	0.83	0.70	0.84	0.89	0.55	1.45	11.53	6.94	2.11	0.59	0.47	1.16	2.34
2004	1.18	2.44	0.90	1.13	2.51	8.67	10.40	14.34	9.39	1.47	0.98	0.99	4.53
2005	0.38	0.42	0.94	0.87	1.33	3.56	14.74	12.47	4.68	1.18	0.89	0.42	3.49
2006	4.35	4.96	2.44	1.04	2.12	3.00	15.31	13.64	2.92	2.65	0.17	0.30	4.41
2007	2.02	4.60	1.35	1.42	1.59	4.14	5.71	21.15	2.19	0.60	0.18	0.24	3.77
2008	0.77	2.29	1.17	0.35	1.36	8.46	9.23	5.97	3.57	1.27	1.04	0.99	3.04
2009	1.51	0.30	0.28	0.36	0.94	3.50	5.98	13.88	5.58	3.63	0.97	1.66	3.22
Average	1.45	1.76	1.12	0.77	1.14	3.00	9.56	11.82	5.31	1.68	0.85	0.77	3.27

Table 4.12 Mean Monthly Discharge Values of 22-208 Station, m<sup>3</sup>/s

	Months												Mean
Years	10	11	12	1	2	3	4	5	6	7	8	9	Annual
2007		1.28	0.56	0.53	0.44	1.15	2.24	3.37	1.36	0.77	0.35	0.33	1.13
2008	0.76	0.90	0.49	0.33	0.37	1.46	1.38	0.59	0.77	0.40	0.43	0.44	0.69
2009	0.94	0.45	0.27	0.40	0.48	1.57	2.07	3.04	2.77	2.99	0.89	1.06	1.41
Average	0.76	1.09	0.53	0.43	0.41	1.30	1.81	1.98	1.06	0.59	0.39	0.38	1.08

Table 4.13 Mean Monthly Discharge Values of 22-222 Station, m<sup>3</sup>/s

	Months												Mean
Years	10	11	12	1	2	3	4	5	6	7	8	9	Annual
2007										0.35	0.09	0.04	0.16
2008	0.09	0.12	0.04	0.04	0.03	0.17	1.59	0.83	0.47	0.25	0.11	0.15	0.33
2009	0.22	0.07	0.05	0.04	0.03	0.06	0.22	3.62	2.73	0.79	0.29	0.32	0.70
Average	0.16	0.10	0.05	0.04	0.03	0.11	0.91	2.22	1.60	0.47	0.16	0.17	0.40

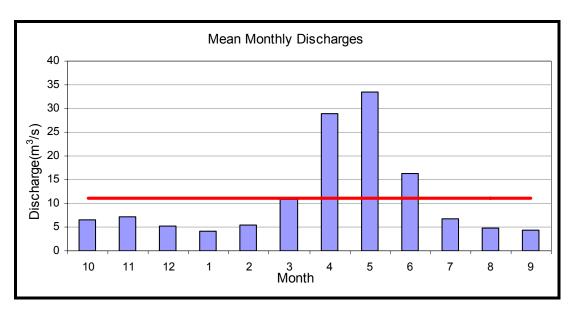


Figure 4.7 Mean Monthly Discharge Values of 2202 Ağnas

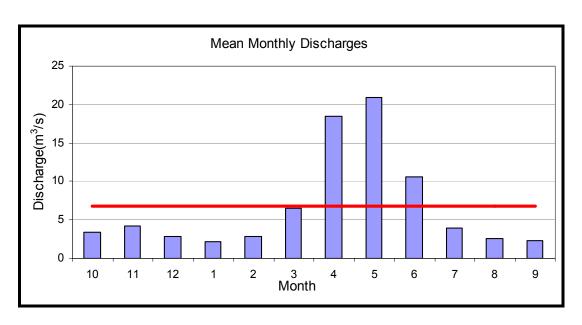


Figure 4.8 Mean Monthly Discharge Values of 22-44 Aytaş

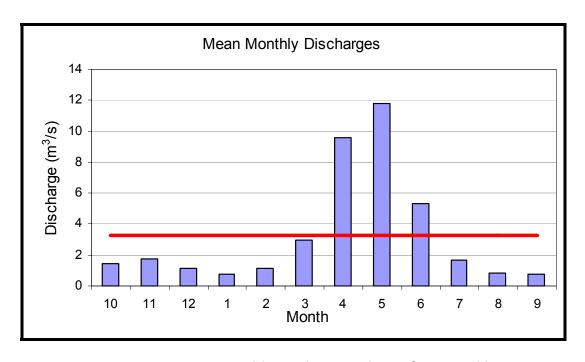


Figure 4.9 Mean Monthly Discharge Values of 2234 Erikli

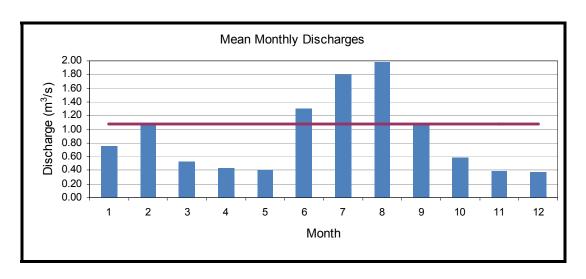


Figure 4.10 Mean Monthly Discharge Values of Station 22-208

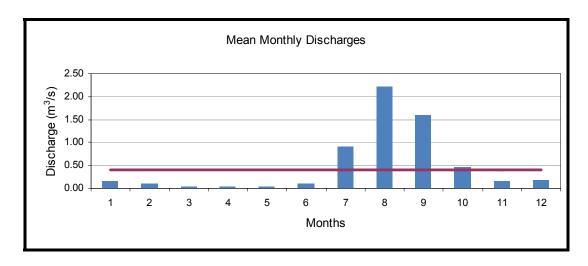


Figure 4.11 Mean Monthly Discharge Values of Station 22-222

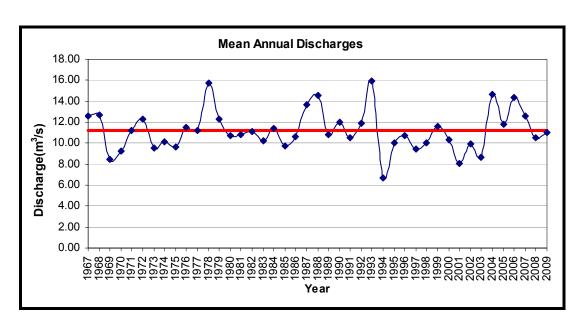


Figure 4.12 Mean Annual Discharge Values of 2202 Ağnas

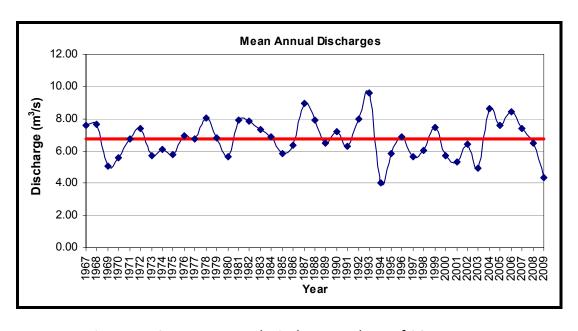


Figure 4.13 Mean Annual Discharge Values of 22-44 Aytaş

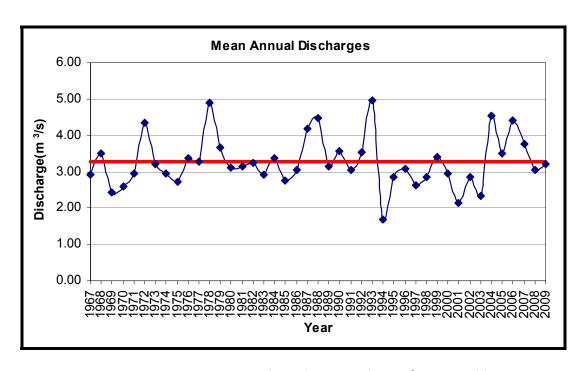


Figure 4.14 Mean Annual Discharge Values of 2234 Erikli

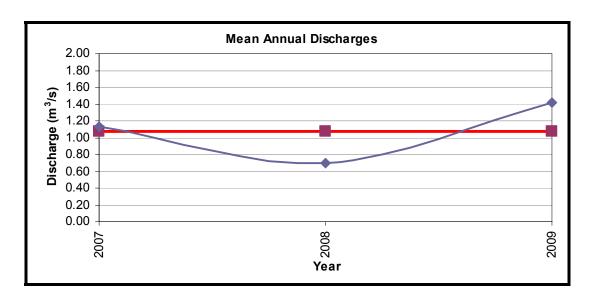


Figure 4.15 Mean Annual Discharge Values of Station 22-208

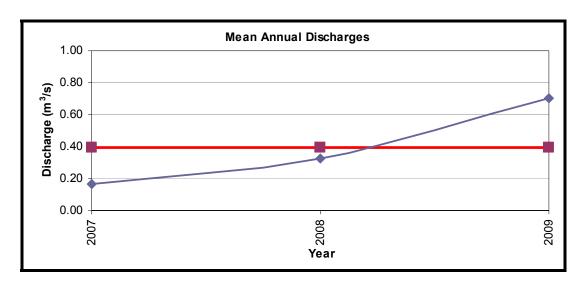


Figure 4.16 Mean Annual Discharge Values of Station 22-222

## 4.3.2 Solaklı Basin

22-52 Ulucami is the station that is the most downstream one; 22-57 Alçakköprü and 22-07 Serah are the stations that are in the upstream part.

Firstly, the stream flow periods of the stations used to obtain flow duration curves are determined.

In Solaklı Basin; for 22-52 Ulucami; the period between 1979 and 2009 is selected. There are not any data in the year 1998. Since there are not also any data within the stations in the basin; the station 2202 Ağnas which is the only station among all stations that has data in 1998 is used to complete the missing values of 22-52 Ulucami. Measurements for 22-57 Alçakköprü started in 1979 and there are measurements till 2005. Measurements in 22-57

Alçakköprü are not continuous but they are completed by regression equation with 22-52 Ulucami being the independent variable. The last station 22-07 Serah started measuring discharge at 1966 till 2007 but it is also a discontinuous station. Its missing data are also completed from 22-52 Ulucami. In addition, for 22-07 Serah the data before 1979 are disregarded.

Before completing the missing values of 22-57 Alçakköprü and 22-07 Serah; the discharge records in all stations are observed; it is seen that some of the records of the sum of 22-57 Alçakköprü and 22-07 Serah are higher than that of 22-52 Ulucami. This is physically impossible since 22-52 Ulucami locates more downstream of 22-57 Alçakköprü and 22-07 Serah (See Figure 3.2). To correct these errors; the values of 22-52 Ulucami which is in the most downstream are accepted to be more reliable.

Firstly, the discharge values of 22-52 Ulucami and the sum of discharge values of 22-57 Alçakköprü and 22-07 Serah are compared and the values of 22-57 Alçakköprü and 22-07 Serah are corrected if the sums of their values are greater. The following equations were used to correct the values:

$$Q_{22-57\text{Alcakkopru}} = Q_{22-52\text{Ulucami}} \left( \frac{A_{22-57\text{Alcakkopru}}}{A_{22-52\text{Ulucami}}} \right)$$
(4.6)

$$Q_{22-07\,\text{Serah}} = Q_{22-52\text{Ulucami}} \left( \frac{A_{22-07\text{Serah}}}{A_{22-52\text{Ulucami}}} \right) \tag{4.7}$$

Missing values in 1998 for 22-52 Ulucami is completed from 2202 Ağnas. The regression equation and its determination coefficient are given in Figure 4.17. Also missing values of 22-57 Alçakköprü and 22-07 Serah are completed by using the values of 22-52 Ulucami via regression equation. The equations for 22-52 Ulucami-22-57 Alçakköprü and 22-52 Ulucami – 22-07 Serah are given below in Figure 4.18 and 4.19 respectively.

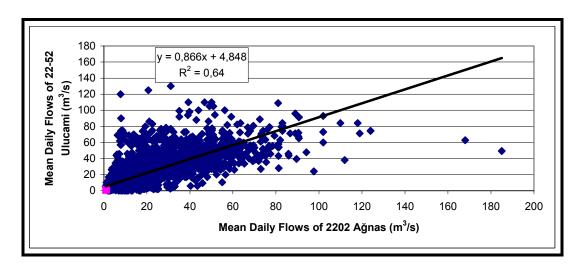


Figure 4.17 Regression Equation of 2202 Ağnas and 22-52 Ulucami

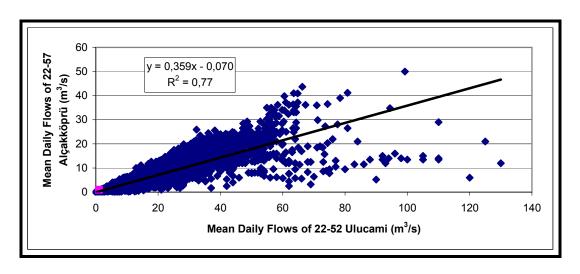


Figure 4.18 Regression Equation of 22-52 Ulucami and 22-57 Alçakköprü

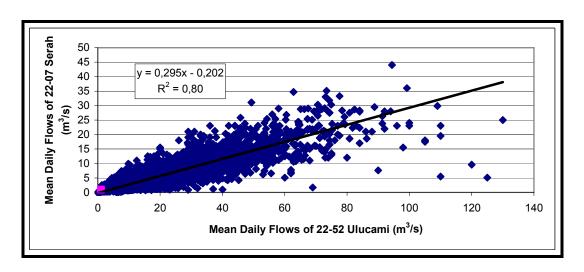


Figure 4.19 Regression Equation of 22-52 Ulucami and 22-07 Serah

The mean monthly discharge values of 22-52 Ulucami, 22-57 Alçakköprü and 22-07 Serah are given below in Table 4.14, 4.15 and 4.16 respectively. Please note that in Table 4.14 the bold values are completed from 2202 Ağnas. Also in 4.15 and 4.16 the bold values are the values that are completed from 22-52 Ulucami by using the daily flows. The mean monthly values of 22-52

Ulucami, 22-57 Alçakköprü and 22-07 Serah for each year are demonstrated in Figure 4.20, 4.21 and 4.22 respectively. The mean annual flows of 22-52 Ulucami, 22-57 Alçakköprü and 22-07 Serah for each year are given in Figure 4.23, 4.24 and 4.25 respectively.

Table 4.14 Mean Monthly Discharge Values of 22-52 Ulucami, m<sup>3</sup>/s

Years	Months								Mean				
	10	11	12	1	2	3	4	5	6	7	8	9	Annual
1979	4.70	8.33	8.73	7.19	7.39	11.06	32.87	45.42	27.40	18.65	7.43	5.62	15.40
1980	9.03	18.09	9.88	5.93	5.74	16.67	37.08	40.06	15.82	3.75	1.29	1.05	13.70
1981	1.64	6.45	4.00	1.62	1.25	5.23	17.11	34.60	45.57	27.53	13.48	11.74	14.18
1982	8.53	16.82	11.69	8.42	7.06	12.55	41.50	39.61	20.90	11.61	5.10	2.39	15.51
1983	3.06	4.21	2.50	1.91	2.89	11.43	22.28	37.45	24.40	11.63	7.09	7.73	11.38
1984	13.97	21.63	8.51	4.20	3.30	8.10	7.21	26.42	25.57	17.02	15.06	5.46	13.04
1985	3.65	2.66	1.71	2.16	2.65	6.70	25.02	34.16	20.35	11.62	3.33	2.56	9.71
1986	10.46	7.82	8.27	5.68	7.71	9.31	24.00	32.74	31.32	12.26	6.06	5.99	13.47
1987	6.57	8.69	7.67	7.61	7.14	6.84	20.31	34.77	24.15	15.67	12.17	7.84	13.29
1988	7.26	10.76	6.81	6.95	7.39	8.61	26.81	48.58	71.53	37.74	34.76	16.33	23.63
1989	17.07	23.04	11.22	4.98	5.62	20.87	52.22	36.38	30.67	12.07	6.53	6.15	18.90
1990	15.07	7.97	11.01	4.13	6.00	16.93	37.09	65.93	35.53	15.49	7.86	6.66	19.14
1991	11.85	17.79	8.39	5.75	7.60	15.65	31.49	41.00	31.36	13.95	8.50	5.53	16.57
1992	5.26	6.45	4.19	3.53	5.39	14.04	32.09	54.15	43.69	19.24	10.98	8.26	17.27
1993	12.80	11.92	10.84	7.65	6.55	13.26	32.04	51.79	38.40	11.88	6.46	6.87	17.54
1994	2.98	4.62	5.63	6.29	6.11	8.87	30.31	27.64	18.93	10.03	7.66	6.98	11.34
1995	4.68	7.52	7.00	7.76	6.11	7.15	20.33	45.25	28.26	15.87	8.97	8.38	13.94
1996	17.24	16.91	9.62	6.12	6.09	5.79	14.19	37.45	19.15	10.12	8.19	9.91	13.40
1997	16.91	8.85	6.29	5.22	6.10	6.83	36.11	43.79	22.82	11.22	7.16	9.38	15.06
1998	13.12	11.19	7.36	8.62	10.68	13.61	25.07	29.90	16.82	8.66	10.15	7.17	13.53
1999	12.21	11.38	11.01	5.56	5.86	8.44	19.71	38.85	25.16	14.68	8.52	7.90	14.11
2000	5.62	5.81	5.83	4.67	6.48	12.14	36.92	23.65	18.60	6.81	5.90	7.07	11.62
2001	12.14	7.80	5.33	4.13	4.44	12.30	24.29	28.43	22.44	10.71	5.81	3.54	11.78
2002	4.00	10.50	10.50	6.66	6.15	9.83	22.98	28.49	34.45	14.58	11.41	8.93	14.04
2003	6.76	5.55	4.81	5.95	3.90	4.92	27.37	29.12	14.01	7.37	5.68	7.55	10.25
2004	7.51	13.36	7.47	6.47	9.70	24.23	27.83	45.62	37.37	15.19	9.15	8.01	17.66
2005	5.46	6.01	7.59	6.78	7.28	10.87	30.94	35.66	24.07	13.51	6.66	6.94	13.48
2006	13.07	18.52	9.71	6.12	7.07	13.16	30.76	38.51	20.40	14.14	8.07	6.66	15.52
2007	8.15	15.31	7.44	6.80	6.43	13.57	22.77	65.41	19.68	11.88	7.07	4.42	15.74
2008	4.18	8.73	6.54	4.23	5.36	22.58	28.29	23.71	20.78	11.37	8.24	6.44	12.54
2009	10.43	5.83	4.42	4.90	5.65	12.64	17.45	38.89	25.59	14.82	7.45	9.96	13.17
Average	8.88	10.66	7.48	5.61	6.03	11.75	27.56	38.82	27.59	13.91	8.78	7.08	14.51

Table 4.15 Mean Monthly Discharge Values of 22-57 Alçakköprü, m³/s

Years						Moi	nths						Mean
	10	11	12	1	2	3	4	5	6	7	8	9	Annual
1979	1.38	2.24	2.61	2.66	2.26	3.80	10.39	17.29	10.00	7.69	3.04	1.52	5.41
1980	2.47	7.27	3.57	2.30	2.48	5.57	13.26	15.89	6.82	1.61	0.55	0.45	5.19
1981	0.74	3.06	1.72	0.70	0.54	2.18	7.38	14.28	15.23	5.50	2.09	1.23	4.55
1982	1.30	3.21	2.43	1.73	1.46	3.19	14.29	12.71	6.84	3.03	1.25	0.64	4.34
1983	0.77	0.69	0.56	0.59	0.58	2.82	6.65	11.16	5.86	1.10	2.26	1.94	2.91
1984	4.95	7.70	2.98	1.44	1.12	2.84	2.52	9.41	9.11	6.04	5.34	1.89	4.61
1985	1.43	1.13	0.73	0.90	0.88	2.38	8.44	12.44	6.41	2.36	1.53	1.07	3.31
1986	3.69	2.74	2.90	1.97	2.70	3.27	8.55	11.68	11.17	4.33	2.11	2.08	4.77
1987	1.72	2.66	2.17	1.81	2.05	2.07	8.31	14.65	10.82	6.58	2.54	1.65	4.75
1988	1.26	4.08	2.75	2.19	1.81	1.94	8.52	12.27	10.76	4.51	3.50	3.14	4.73
1989	7.36	11.26	4.18	1.88	1.98	7.16	27.83	19.01	9.82	3.27	1.62	1.34	8.06
1990	5.34	2.79	3.88	1.41	2.08	6.01	13.25	23.60	12.69	5.49	2.75	2.32	6.80
1991	3.65	6.17	2.73	1.36	1.68	5.24	14.57	17.49	10.00	3.58	2.12	1.74	5.86
1992	1.71	3.17	1.88	1.55	1.66	3.71	12.70	25.23	15.24	5.10	2.52	2.57	6.42
1993	4.52	4.21	3.82	2.68	2.28	4.69	11.43	18.52	13.71	4.19	2.25	2.40	6.23
1994	1.38	1.86	2.55	1.58	1.77	3.47	13.93	12.72	6.59	2.76	1.84	1.49	4.33
1995	2.02	3.21	2.91	3.14	2.76	3.80	10.29	26.53	11.86	3.48	2.05	2.01	6.17
1996	5.49	6.35	3.20	1.92	1.74	1.75	5.81	17.13	7.90	3.42	2.62	3.06	5.03
1997	6.00	3.11	2.19	1.80	2.12	2.38	12.89	15.65	8.12	3.96	2.50	3.30	5.34
1998	4.64	3.95	2.57	3.03	3.76	4.82	8.93	10.66	5.97	3.04	3.58	2.51	4.79
1999	1.69	2.95	4.82	1.67	1.28	2.73	11.52	19.85	11.49	5.10	2.43	2.32	5.65
2000	1.75	2.14	2.20	1.30	2.41	3.85	19.26	13.80	9.71	2.94	1.17	1.40	5.16
2001	4.95	4.06	1.66	1.15	1.07	4.89	11.97	13.87	8.40	2.53	1.19	0.58	4.69
2002	0.65	2.23	1.33	1.07	2.08	4.44	9.14	12.90	11.01	4.24	2.18	2.27	4.46
2003	2.21	2.40	2.08	2.17	1.39	1.80	11.57	12.14	4.99	1.82	1.15	1.28	3.75
2004	1.53	4.98	2.39	1.74	2.24	9.22	11.24	19.53	16.44	5.68	2.40	2.09	6.62
2005	1.36	1.62	1.97	1.80	1.83	4.10	14.36	15.91	10.23	5.62	2.41	2.16	5.28
2006	4.62	6.58	3.41	2.13	2.47	4.65	10.97	13.75	7.25	5.01	2.83	2.32	5.50
2007	2.86	5.42	2.60	2.37	2.24	4.80	8.11	23.41	7.00	4.20	2.47	1.52	5.58
2008	1.43	3.06	2.28	1.45	1.85	8.04	10.09	8.44	7.39	4.01	2.89	2.24	4.43
2009	3.68	2.02	1.52	1.69	1.96	4.47	6.19	13.89	9.12	5.25	2.61	3.51	4.66
Average	2.86	3.82	2.54	1.78	1.89	4.07	11.11	15.67	9.61	4.11	2.32	1.94	5.14

Table 4.16 Mean Monthly Discharge Values of 22-07 Serah, m<sup>3</sup>/s

Years						М	onths						Mean
	10	11	12	1	2	3	4	5	6	7	8	9	Annual
1979	1.76	1.91	1.82	1.22	1.33	1.97	6.09	13.25	11.36	6.93	2.31	1.52	4.29
1980	1.91	3.73	2.40	1.44	1.87	3.97	8.56	14.36	5.02	1.00	0.34	0.28	3.74
1981	0.44	1.43	1.06	0.43	0.33	1.43	5.24	10.32	16.78	8.45	3.08	1.97	4.25
1982	2.31	4.76	3.25	2.28	1.88	3.50	12.04	11.48	5.96	3.22	1.30	0.50	4.37
1983	0.94	1.69	1.22	0.62	0.97	1.37	8.15	11.05	8.56	4.24	1.79	1.38	3.50
1984	2.43	3.61	1.52	0.79	0.36	1.18	3.23	10.66	9.80	5.58	2.44	1.78	3.61
1985	0.70	0.65	0.46	0.54	0.43	1.04	5.33	11.53	7.19	2.47	1.05	0.70	2.67
1986	2.38	2.00	1.59	1.24	1.36	1.98	5.92	9.23	15.48	6.65	2.01	1.25	4.26
1987	1.65	2.03	1.55	1.57	1.75	1.50	4.52	11.51	8.96	5.78	3.49	1.76	3.84
1988	1.64	2.51	1.56	1.31	1.46	1.09	6.38	13.25	17.60	11.47	5.50	3.05	5.57
1989	4.51	4.29	2.73	1.38	1.49	4.75	14.10	12.15	12.33	5.70	2.24	1.28	5.58
1990	3.28	1.94	1.94	1.10	1.16	2.91	8.99	17.73	13.69	6.64	2.37	1.38	5.26
1991	1.96	3.32	1.63	1.09	1.11	3.23	7.95	12.39	12.30	5.55	2.28	1.18	4.50
1992	1.70	2.11	1.28	1.01	0.97	2.05	6.89	14.85	17.49	7.31	3.30	2.09	5.09
1993	3.53	2.71	1.90	1.37	1.32	2.47	8.35	17.48	17.19	7.77	3.15	1.44	5.72
1994	0.80	1.28	1.40	1.03	1.00	1.98	9.14	10.34	6.52	4.34	2.14	1.29	3.44
1995	1.18	2.02	1.86	2.09	1.60	1.91	5.80	13.15	8.14	4.48	2.45	2.27	3.91
1996	4.14	4.48	2.28	1.57	1.20	1.20	3.41	12.24	7.80	4.81	2.21	2.05	3.95
1997	4.32	2.58	1.77	1.48	1.35	1.55	7.86	13.83	9.82	5.21	2.15	2.22	4.51
1998	3.67	3.10	1.97	2.34	2.95	3.81	7.19	8.62	4.76	2.35	2.79	1.91	3.79
1999	1.31	1.41	2.08	0.83	0.81	1.45	4.60	11.69	9.72	5.03	2.02	1.43	3.53
2000	0.91	1.35	1.47	0.96	0.99	1.60	7.73	6.17	6.26	2.19	1.34	1.10	2.67
2001	2.82	1.57	0.97	0.87	0.88	3.13	5.75	8.32	8.84	3.67	1.26	0.94	3.25
2002	1.24	1.35	0.89	0.85	1.34	2.18	5.37	9.08	14.13	6.04	2.06	2.08	3.88
2003	2.06	1.58	1.12	1.20	0.89	0.91	6.73	10.24	6.88	3.09	2.61	2.93	3.35
2004	2.10	3.24	1.61	1.15	1.30	4.44	5.43	12.68	12.58	5.17	2.18	1.45	4.44
2005	1.36	1.29	1.26	1.30	0.98	1.90	7.19	9.89	6.40	4.48	2.70	2.51	3.44
2006	5.16	4.92	2.63	1.11	1.17	2.65	6.83	11.86	8.27	4.57	1.11	0.26	4.21
2007	1.85	4.25	0.93	0.96	1.20	3.48	2.25	23.34	9.43	3.73	1.49	1.39	4.53
2008	1.03	2.37	1.73	1.05	1.38	6.46	8.14	6.79	5.93	3.15	2.23	1.70	3.50
2009	2.88	1.52	1.10	1.24	1.47	3.53	4.95	11.27	7.35	4.17	2.00	2.74	3.68
Average	2.19	2.48	1.64	1.21	1.24	2.47	6.78	11.96	10.08	5.01	2.24	1.61	4.08

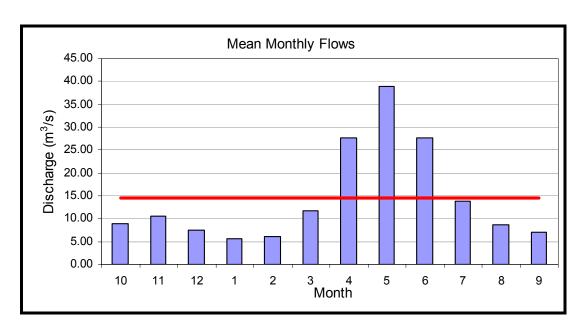


Figure 4.20 Mean Monthly Discharge Values of 22-52 Ulucami

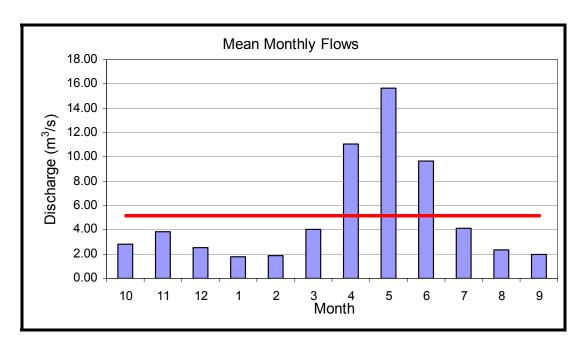


Figure 4.21 Mean Monthly Discharge Values of 22-57 Alçakköprü

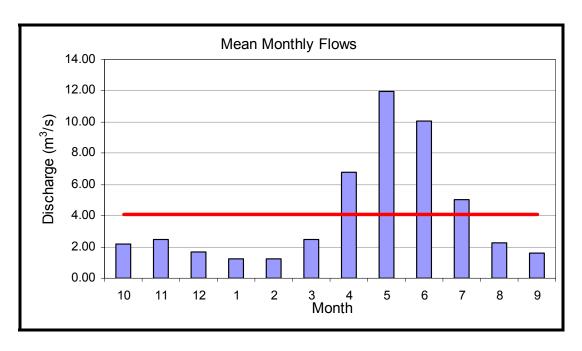


Figure 4.22 Mean Annual Discharge Values of 22-07 Serah

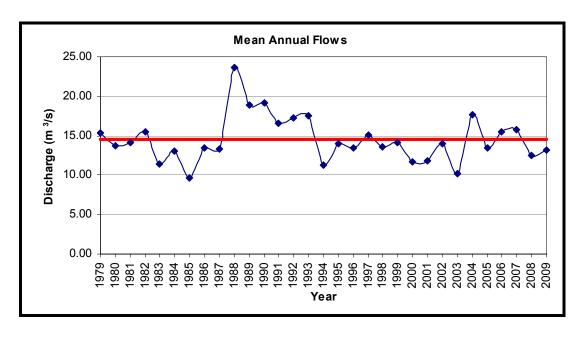


Figure 4.23 Mean Annual Discharge Values of 22-52 Ulucami

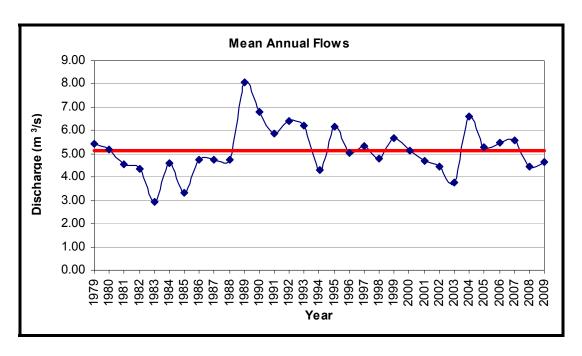


Figure 4.24 Mean Annual Discharge Values of 22-57 Alçakköprü

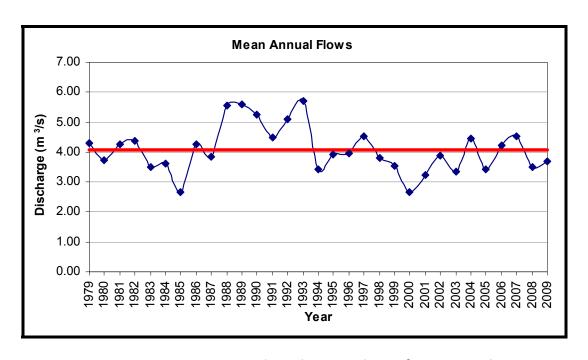


Figure 4.25 Mean Annual Discharge Values of 22-07 Serah

The mean annual discharge values of the flow gauging stations in Solaklı and Karadere basins are given in Table 4.17.

Table 4.17 Mean Annual Discharge/Specific Discharge Values of the Streamflow Gauging Stations within Karadere and Solaklı Basins

OL C. N	Mean Annual Discharge Values	Drainage Area	Number of Records
Station No	(m³/s)	(km <sup>2</sup> )	(Year)
2202	11.18	634.77	43
22-44	6.74	425.90	43
2234	3.27	206.79	43
22-52	14.51	563.22	31
22-57	5.14	240.76	31
22-007	4.08	148.72	31
22-208	1.08	38.73	3
22-222	0.4	9.22	3

## **4.3.3 Flow Duration Curves**

According to Cigizoglu et al. (2000); flow duration curve (FDC) is a representation of the relationship between the magnitude and the frequency of either daily, monthly, weekly or some other time interval of stream flows for a particular river basin , providing an estimate of the percentage of time the stream flow was equalled or exceeded over a historical period. FDCs are

generally used in water resources applications and recently they have been used in validating the outputs of hydrologic models and/or compare observed and modeled hydrologic response (Post, 2004).

In this study both annual and seasonal flow duration curves of the related stream flow gauging stations are derived. In the first part, annual and in the second part seasonal (spring) flow duration curves are presented.

The flow duration curves are necessary to determine the flow values corresponding to 8 selected flow percentiles varying from 5% to 40% with a 5% increment. The effect of the parameters to each flow percentile are going to be investigated and 8 different models seasonally and annually are going to be set up. The necessary information about the models is presented in Chapter 5.

In the following sections; the annual and seasonal FDCs of each flow gauging stations are given. These flow duration curves are obtained by running the macro program "Su Temini". The FDCs of 22-208 and 22-222 which are individual gauging stations operated by private companies are not given since their period of observation is very short (2 years).

### 4.3.3.1 Annual Flow Duration Curves

The annual flow duration curves of 2202 Ağnas, 22-44 Aytaş, 2234 Erikli, 22-52 Ulucami, 22-57 Alçakköprü and 22-07 Serah are given below in between Figure 4.26 and 4.31:

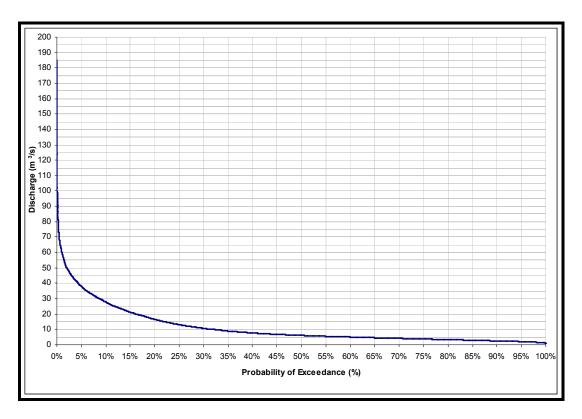


Figure 4.26 Annual Flow Duration Curve of 2202 Ağnas

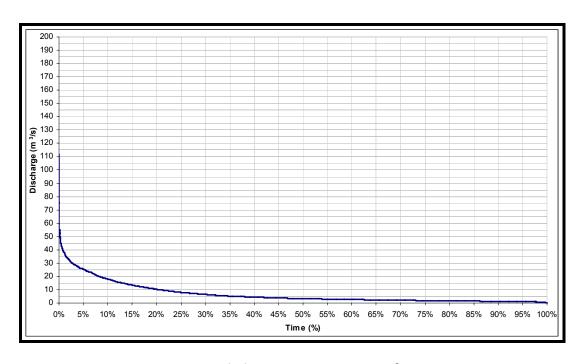


Figure 4.27 Annual Flow Duration Curve of 22-44 Aytaş

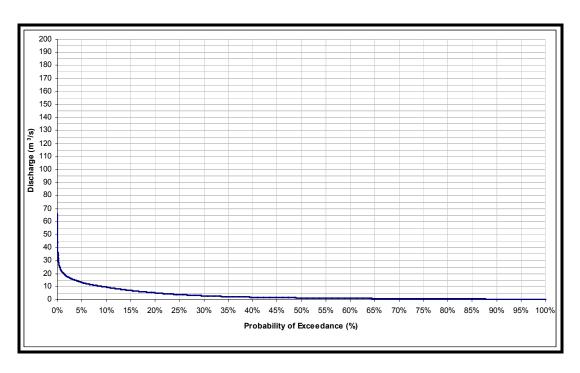


Figure 4.28 Annual Flow Duration Curve of 2234 Erikli

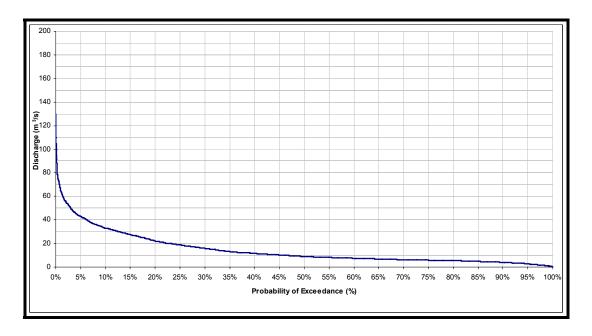


Figure 4.29 Annual Flow Duration Curve of 22-52 Ulucami

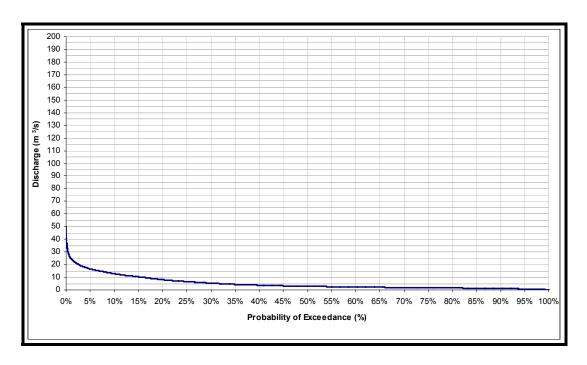


Figure 4.30 Annual Flow Duration Curve of 22-57 Alçakköprü

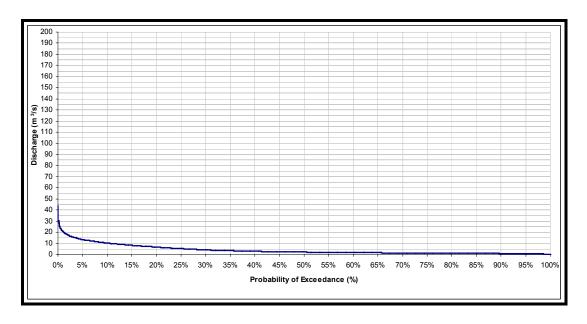


Figure 4.31 Annual Flow Duration Curve of 22-07 Serah

## 4.3.3.2 Seasonal Flow Duration Curves

Seasonal flow duration curves are required in order to set up spring season model. These curves are derived by considering only the spring season (march, april and may months) of the year by disregarding the rest of the year. Using the same macro program, the seasonal flow duration curves are obtained. The seasonal flow duration curves of 2202 Ağnas, 22-44 Aytaş, 2234 Erikli, 22-52 Ulucami, 22-57 Alçakköprü and 22-07 Serah are given below in between Figure 4.32 and 4.37:

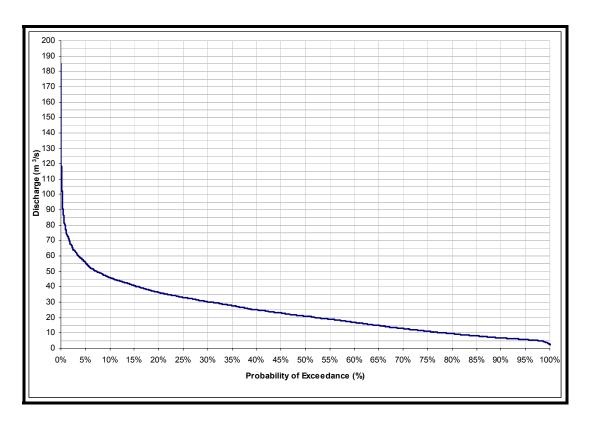


Figure 4.32 Seasonal Flow Duration Curve of 2202 Ağnas

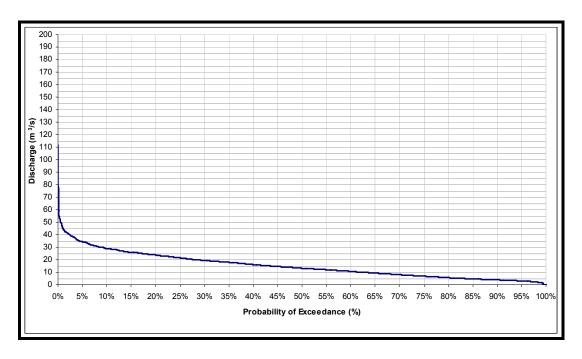


Figure 4.33 Seasonal Flow Duration Curve of 22-44 Aytaş

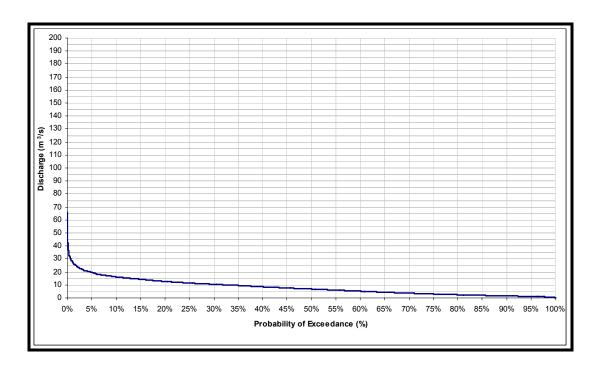


Figure 4.34 Seasonal Flow Duration Curve of 2234 Erikli

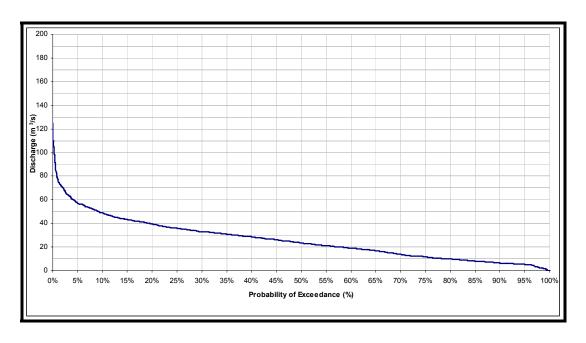


Figure 4.35 Seasonal Flow Duration Curve of 22-52 Ulucami

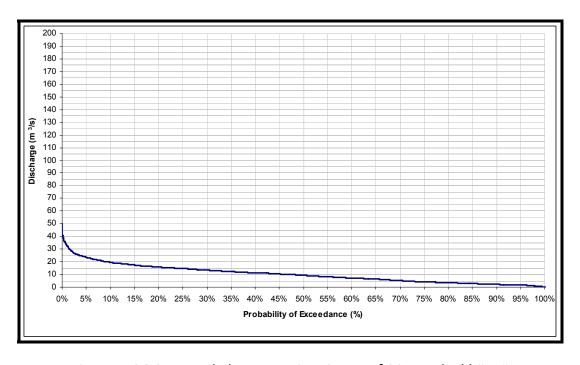


Figure 4.36 Seasonal Flow Duration Curve of 22-57 Alçakköprü

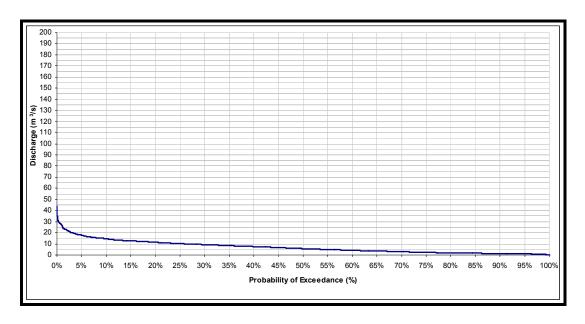


Figure 4.37 Seasonal Flow Duration Curve of 22-07 Serah

# 4.3.4 Estimation of Project Runoffs at Project Sites

Since there are only 6 stream flow gauging stations available for the model and this number is not enough to set up a statistical model, more basins are needed to increase the number of degrees of freedom in the model. For this reason, the locations and some other properties of some planned small HEPP projects within Solaklı and Karadere basins (see Table 3.3 and 3.4) are gathered and the parameters of these basins are extracted. To find out the daily stream flow series of these basins, there are some methods to transfer stream flow from gauged basins to ungauged basins.

Although in literature there are methods on drainage area ratio methods; a different regional approach specifically for the study area is applied in order to look for a relationship between drainage areas and flow values corresponding to different percentiles. The regional relationship between drainage area and flow value is more advantageous than conventional

drainage area ratio methods as the predicted flow values bring less intercorrelation between variables in the statistical models that are going to be explained in Chapter 5. Besides this, being a regional method; in other words; the relationships for each percentile are specific for the study area, the predicted flow values would be more realistic.

As stated in Chapter 4.1.3.3, the FDCs are divided into 8 parts and each part is modeled separately. To determine the flow values for each percentile for the project sites; 8 separate relationships are derived for the annual model. Also, 8 relationships are derived for the seasonal (spring) models.

In order to derive the relationships between drainage area and flow values, 11 flow gauging stations are used. 3 flow gauging stations are located in Karadere basin which are 2202, 22-44 and 2234. 3 flow gauging stations locate in Solaklı Basin which are 22-52, 22-57 and 22-07, and 5 flow gauging stations are in İyidere Basin which is not in the study area but adjacent to Solaklı Basin. The IDs of flow gauging stations in İyidere Basin are 2218, 2233, 22-78, 2296 and 2215. The information about the gauging stations in İyidere Basin is gathered from an ongoing research.

For a flow value of a gauging station corresponding to a certain exceedance probability, the drainage area of that gauging station is determined. Then, the points are plotted and a relationship is derived for a certain flow percentile. The regression analyses are performed for each flow percentile and 8 regression equations are formed. The equations and their respective determination coefficients are given below in Table 4.18. Here y stands for

discharge value ( $m^3/s$ ) corresponding to the related percentile, whereas x stands for drainage area ( $km^2$ ). Also in Figure 4.38 and 4.39, relationships for annual and seasonal (spring) flows could be seen.

Table 4.18 The Relationships between Drainage Area and Related Discharges

	Annual Relationship	Seasonal Relationship
Relationship for 5% of FDC	y=0.2573x <sup>0.8038</sup> ; R <sup>2</sup> =0.73	y=0.5279x <sup>0.7337</sup> ; R <sup>2</sup> =0.72
Relationship for 10% of FDC	y=0.1336x <sup>0.8647</sup> ; R <sup>2</sup> =0.75	y=0.2744x <sup>0.8104</sup> ; R <sup>2</sup> =0.79
Relationship for 15% of FDC	y=0.0769x <sup>0.9183</sup> ; R <sup>2</sup> =0.76	y=0.1748x <sup>0.8611</sup> ; R <sup>2</sup> =0.83
Relationship for 20% of FDC	y=0.0492x <sup>0.9540</sup> ; R <sup>2</sup> =0.76	y=0.1300x <sup>0.8923</sup> ; R <sup>2</sup> =0.85
Relationship for 25% of FDC	y=0.0328x <sup>0.9828</sup> ; R <sup>2</sup> =0.77	y=0.1030x <sup>0.9136</sup> ; R <sup>2</sup> =0.87
Relationship for 30% of FDC	y=0.0216x <sup>1.0162</sup> ; R <sup>2</sup> =0.78	y=0.0790x <sup>0.9419</sup> ; R <sup>2</sup> =0.89
Relationship for 35% of FDC	y=0.0150x <sup>1.0667</sup> ; R <sup>2</sup> =0.78	y=0.0582x <sup>0.9765</sup> ; R <sup>2</sup> =0.91
Relationship for 40% of FDC	y=0.0111x <sup>1.0675</sup> ; R <sup>2</sup> =0.77	$y=0.0464x^{0.9972}$ ; $R^2=0.92$

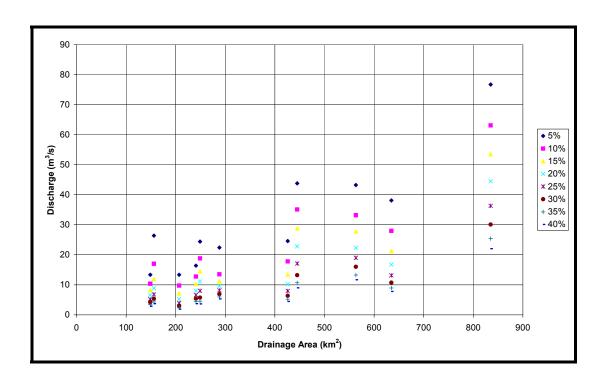


Figure 4.38 Regional Relationship of Flow Gauging Stations for Annual Flows

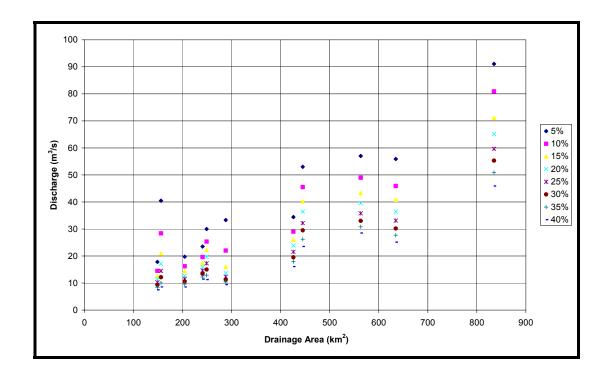


Figure 4.39 Regional Relationship of Flow Gauging Stations for Seasonal Flows

In Figure 4.38 and 4.39 and in Table 4.18; it can be concluded that the relationships between 11 flow gauging stations are good. The ranges for determination coefficients vary from 0.70 to 0.93.

The flow values of facility sites corresponding to each selected exceedance probability could be calculated depending on drainage areas of the facility sites.

## 4.4 Topographic Data

ASTER DEM products are used in a resolution of 30x30 m as stated in Chapter 3.2.3. By using the Arc Hydro extension of Arc GIS, some basin characteristics are derived after obtaining the digital elevation models. The Arc Hydro tools are used to derive several data sets that collectively describe the drainage patterns of a catchment. Raster analysis is performed to generate data on flow direction, flow accumulation, stream definition, stream segmentation and watershed delineation. These data are then used to develop a vector representation of catchments and drainage lines. Using this information, a geometric network is constructed.

The whole steps in delineating the watershed are described in the flowchart given in Figure 4.40.

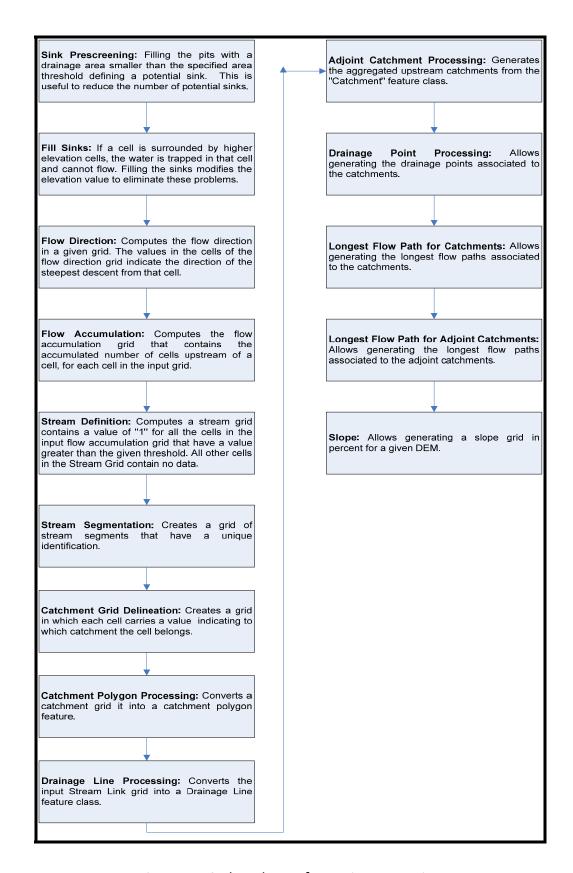


Figure 4.40 Flowchart of Terrain Processing

The terrain preprocessing is the stage just before delineating the watershed. It uses DEM to identify the surface drainage pattern. The steps in this stage are performed sequentially. The steps after terrain processing are under the watershed processing stage which is described below:

Firstly, the outlet points for the watersheds are determined in order to get the characteristics of the watershed. The watersheds of the stream gauging stations within Karadere and Solaklı watersheds together with the watersheds of the diversion weirs of hydro-power facilities are delineated. The coordinate information for the gauging stations is gathered from DSI and EIEI; some important properties of the diversion weirs of the HEPPs are gathered from DSI by personal communication as explained in Chapter 3.2.5. Also the subwatershed delineation is performed accordingly. After then, drainage area centroids and longest flow paths of the related watersheds are determined.

### 4.5 Snow Covered Area Data

SEVIRI SR product images for the months of January-May in 2008 and 2009 are obtained (S.Surer, personal communication, May 16 2010). as well as the mean daily temperature values of Uzungöl DMI. In this part; the aim is to look for a relationship between mean daily temperature values of Uzungöl DMI and snow covered area data of each basin.

The snow covered area data of Solaklı and Karadere basins are determined by using the "raster calculator" tool. Multiplying the related watershed polygon

with the raster of SEVIRI image file of a particular day, a new map is obtained representing the snow covered area of the related basin. Some portions of the basin can either be covered with snow or/and clouds. This means that the rest of the basin is not covered with clouds or snow. The total area of the snow is calculated by counting the number of pixels that are under snow and multiplying it with the spatial resolution. Knowing the total area of the basin, the ratio of snow area to the total basin area yields snow covered area as percentage. This procedure is performed for each day that are available. One should note that the maps containing more than 25% of clouds are disregarded and the rest of the maps are considered for the analyses. Totally, 35 days from 2009 and 21 days from 2008 year are studied. January month is not used in these analyses because the snowmelt did not start in January month.

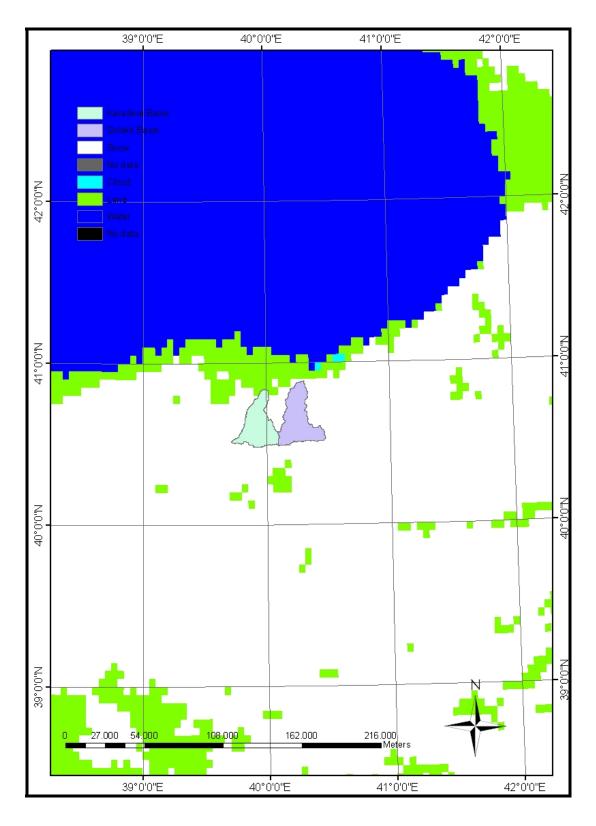


Figure 4.41 Karadere and Solaklı Basins on 14 January 2008

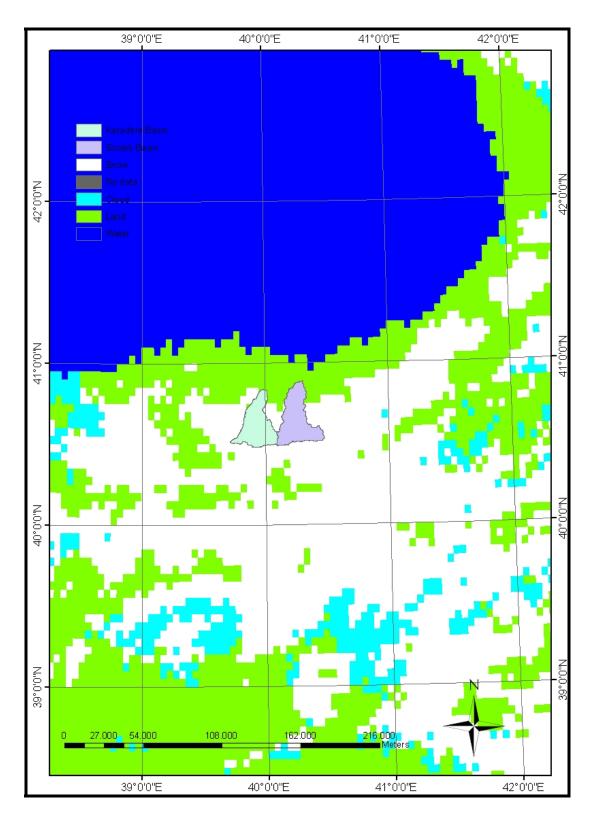


Figure 4.42 Karadere and Solaklı Basins on 12 March 2008

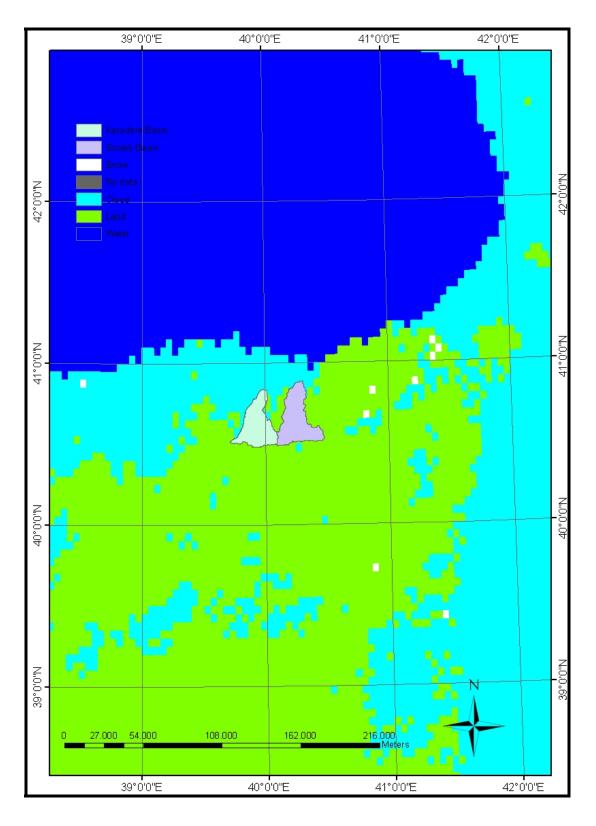


Figure 4.43 Karadere and Solaklı Basins on 29 April 2008

In Figures between 4.41-4.43, the situations of the basins on 14 January 2008, 12 March 2008 and 29 April 2008 can be seen respectively. If one can compare Figure 4.41 and 4.42 one can see the melting of snow in the region in one month. In Figure 4.43 a picture from a cloudy day can be seen.

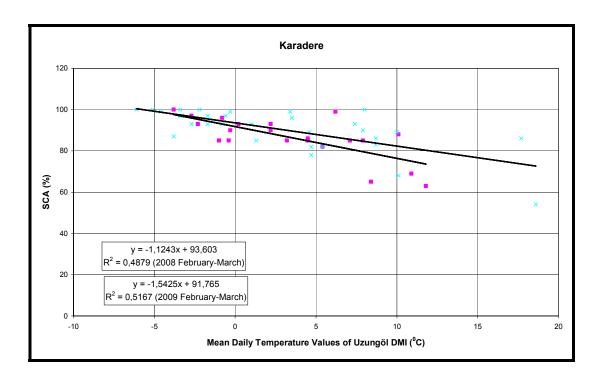


Figure 4.44 SCA-Mean Daily Temperature Values of Uzungöl DMI Relationship for Karadere Basin

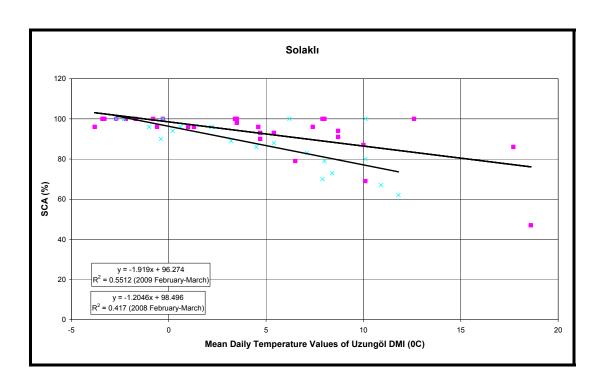


Figure 4.45 SCA-Mean Daily Temperature Values of Uzungöl DMI Relationship for Solaklı Basin

Table 4.19 Summary Table of Correlation Analyses for SCA-Mean Daily

Temperature Values of Uzungöl DMI

	2008 (February-	2009 (February-			
	March)	March)			
I/ a na al a na	y = -1.12x + 93.60,	y = -1.54x + 91.78			
Karadere	$R^2$ =0.49	$R^2$ =0.52			
Cololdi	y = -1.21x + 98.50,	y = -1.92x + 96.27,			
Solaklı	$R^2=0.42$	$R^2$ =0.55			

In Table 4.19 above, the relationship between mean daily temperature values of Uzungöl DMI, in °C, and snow covered area values, in %, can be seen. The

dependent variable, y, stands for snow covered area value of the related basin; whereas the independent variable, x, stands for mean daily temperature value of Uzungöl DMI.

As can be seen from Figure 4.36, 4.37 and Table 4.19; the snowmelt rates in both basins are relatively similar. Furthermore, in 2009 the rate of snowmelt is faster compared to 2008. Furthermore, the snow covered area and mean daily temperature is inversely proportional to each other which is physically rational.

As a result; mean daily temperature values of Uzungöl DMI is accepted to represent snow covered area values of Solaklı and Karadere basins. In this case; mean temperature value of the spring season for each basin can be representative of effect of snowmelting within the seasonal models.

### **CHAPTER 5**

## MODEL DEVELOPMENT AND DISCUSSION OF RESULTS

## 5.1 Introduction

The main objective of this chapter is to set up regional prediction models for FDCs both annually and seasonally (spring season) that are corresponding to 8 flow percentiles (5%, 10%, 15%, 20%, 25%, 30%, 35% and 40%) using some of the parameters from each category which are described in the preceding sections and also the dependent hydrologic variable, specific runoff. Specific runoff values (m³/s/km²) are used instead of flow values (m³/s) by dividing the flow values to drainage area of the basin concerned.

The range from 5% to 40% of the flow duration curves of each basin are selected because the project discharges of facilities those operate for energy production purpose lies in this range. Since flow values for different percentiles of FDCs are affected by different parameters and/or in different degrees, it is decided to model each flow percentile separately.

Besides the annual models; the spring seasonal models are also selected. The reason why spring season is selected for the modeling is to look for the influence of the snowmelt to the flow percentiles of the FDCs of spring season which are usually higher than the other seasons.

The sample size is 16 with 5 of them being flow gauging stations and 11 of them being the HEPP facilities within the project sites. DSI 22-52 Ulucami flow gauging station is used for validation.

### 5.2 Topographic Parameters

All the basins required for the models are delineated and related parameters are extracted and these parameters are grouped in four categories which are linear measures, relief or slope parameters, shape parameters, morphological parameters and hydrological parameters.

The only linear measure is the perimeter of the basin (P); in km; whereas the shape measure is the ratio of the perimeter of the basin to the main stream length of the same basin (P/L) which is a dimensionless unit. This unit is an indicator of the shape of the basin. The smaller P/L values indicate relatively narrower and rectangular basins; on the other hand the bigger P/L values indicate a wider and circular basin shape.

The morphological measures are the drainage density and drainage frequency parameters.

Drainage density,  $D_d$ , of a basin is the total length of all the branches of a river per unit area and it shows how the basin is drained (Usul, 2001). The formula for the drainage density is given below in Equation 5.1:

$$D_d = \frac{\sum L_u}{A} \tag{5.1}$$

Where  $D_d$  is the drainage density in m/km<sup>2</sup>,  $\Sigma L_U$  is the total length of the stream branches of all orders in m, and A is the basin area in km<sup>2</sup>.

A similar term, drainage frequency, D<sub>f</sub>, gives the same information with the number of branches. It is equal to the total number of branches from all orders per unit area. The formula for the drainage frequency is given below in Equation 5.2:

$$D_f = \frac{\sum N_u}{A} \tag{5.2}$$

Precipitation and discharge parameters are calculated as they are stated in Chapter 4. For the seasonal model mean seasonal temperature of the basin (T) is also used in °C. Here the season is selected as spring. Furthermore; for the seasonal model, mean precipitation of the basin for the spring season (MSP) is used instead of annual mean precipitation (MAP).

The parameters for each basin are given in Table 5.1. In Table 5.2 and 5.3; the specific discharge values for annual and seasonal FDCs can be seen respectively.

Table 5.1 The Parameter List of the Basins Used in the Model

CATEGO	CATEGORIES OF PARAMETERS	Linear	Relief/Slope	Relief/Slope	Shape	Morphological	Morphological	Hydro- Meteorological	Hydro- Meteorological	Hydro- Meteorological
NAME OF THE BASIN	PROJECT OR STATION NAME	Basin Perimeter (km)		Mean Slope of Maximum Basin Perimeter/Main Basin (%) Relief (m) Stream Lenght	Perimeter/Main Stream Lenght	Drainage Frequency (km²)	Drainage Density (km <sup>-1</sup> )	Mean Annual Precipitation (mm)	Mean Spring Precipitation (mm)	Mean Spring Temperature (°C)
	Çaykara	161.5	44.85	3077	3.61	0.55	0.64	1816.1	388.6	4.1
	Güneşli-II	179.9	45.43	3191	3.43	0.54	0.64	1746.2	373.6	4.8
	Ballica	194.0	44.94	3263	3.39	0.54	0.64	1703.2	364.4	5.2
NI S	Arca	202.3	44.75	3294	3.28	0.55	0.64	1676.3	358.7	5.5
CTS LI	Uzungöl	83.6	47.96	2373	3.09	0.55	0.64	1659.1	445.2	2.8
TYK O1E	Oğlaklı	61.6	41.79	1845	3.10	0.46	0.63	1580.3	424.1	3.5
	Irmak	67.4	36.91	2105	3.97	09.0	0.67	1639.1	439.8	3.0
	Bangal	74.5	35.08	1186	3.58	0.50	0.63	1168.4	300.4	3.3
	Çanak-l	13.3	25.74	964	2.86	0.64	0.41	1169.7	300.8	3.3
∃LO ⊒AS	Akkocak	2.62	35.49	1418	3.39	0.50	0.62	1166.1	299.8	3.3
NI	Erikli	89.1	36.77	1500	4.06	0.56	0.63	1190.1	306.0	3.0
NI SI	22-57	100.3	40.27	2379	3.74	0.54	0.65	1585.3	425.4	3.4
GAUGIN STATION SOLAKLI	2207	72.4	47.59	2240	2.85	0.56	0.64	1693.4	454.4	2.5
DUS NG NG	2202	178.6	41.56	2755	3.20	0.54	0.63	1122.5	288.6	4.0
	22-44	134.2	38.75	2322	4.03	0.54	0.62	1168.4	300.4	3.3
IN SĽ	2234	88.8	36.65	1503	4.12	0.57	0.63	1190.4	306.1	3.0
:OB	22-52	159.5	44.93	3053	3.62	0.55	0.64	1815.7	388.5	4.1
DITATS 9 G3SU 4GIJAV	22-208	34.6	37.08	2150	2.77	0.54	0.65	1086.7	279.4	4.6

Table 5.2 Specific Discharge Values of Each
Basin Corresponding to Each Exceedance
Probability for Annual FDCs

	2234	0.0638	0.0466	0.0340	0.0251	0.0188	0.0144	0.0111	
ì	22-44	0.0594	0.0424	0.0317	0.0244	0.0188	0.0151	0.0122	0.0103
	2202	0.0599	0.0440	0.0334	0.0263	0.0206	0.0169	0.0141	0.0100
	22-007	0.0901	0.0699	0.0558	0.0444	0.0352	0.0282	0.0229	0.0104
	22-57	0.0685	0.0534	0.0428	0.0336	0.0276	0.0225	0.0184	0.0156
(m³/s/km²)	Erikli	0.0903	0.0649	0.0497	0.0385	0.0299	0.0236	0.0190	0.0150
Percentile (	Akkocak	0.0973	0.0683	0.0513	0.0392	0.0301	0.0234	0.0187	0.0155
Corresponding to Each Percentile (m <sup>3</sup> /s/km	Çanak-1	0.1897	0.1083	0.0677	0.0458	0.0319	0.0222	0.0161	0.0123
Correspond	Bangal	0.0980	0.0687	0.0515	0.0392	0.0301	0.0234	0.0187	0.0155
Specific Flow Values (	Irmak	0.1013	0.0702	0.0522	0.0395	0.0302	0.0233	0.0185	0.0153
Specific F	Oğlaklı	0.1067	0.0728	0.0533	0.0400	0.0304	0.0232	0.0183	0.0150
	Uzungol	0.0943	6990.0	0.0506	0.0389	0.0300	0.0235	0.0189	0.0157
	Arca	0.0705	0.0547	0.0448	0.0363	0.0293	0.0240	0.0201	0.0173
	Ballica	0.0710	0.0550	0.0450	0.0364	0.0293	0.0240	0.0201	0.0173
	Güneşli-2	0.0722	0.0556	0.0453	0.0365	0.0293	0.0240	0.0200	0.0172
	Çaykara	0.0742	0.0567	0.0458	0.0368	0.0294	0.0239	0.0199	0.0170
Flow	Percentile	%9	10%	15%	20%	72%	30%	32%	70UV

Table 5.3 Specific Discharge Values of Each
Basin Corresponding to Each Exceedance
Probability for Seasonal FDCs

	4	99	93	90	21	54	17	71	٦۵
	2234	0.0966	0.0793	0.0706	0.0621	0.0564	0.0517	0.0471	0.0418
	22-44	0.0808	0.0681	0.0610	0.0561	0.0506	0.0458	0.0420	82800
	2202	0.0881	0.0723	0.0643	0.0573	0.0521	0.0476	0.0436	56EU U
	22-007	0.1197	9260'0	0.0847	6.0773	6890'0	0.0627	0.0565	7050 O
	22-57	9260.0	0.0814	0.0723	0990'0	9090'0	0.0557	0.0507	0.0474
(m~/s/km <sup>-</sup> )	Erikli	0.1275	8660'0	0.0833	0.0732	0990'0	6/20.0	0.0513	257U U
Specific Flow Values Corresponding to Each Percentile (m"/s/km'	Akkocak	0.1410	0.1072	0.0878	0.0762	0.0671	0.0592	0.0518	0.0458
ling to Each	Çanak-1	0.3492	0.2044	0.1409	0.1100	0.0901	0.0722	0.0561	0.0462
Correspond	Bangal	0.1425	0.1080	0.0883	0.0765	0.0673	0.0594	0.0518	0.0458
low Values	Irmak	0.1489	0.1115	0.0903	0.0779	0.0683	0.0599	0.0521	0.0458
Specific F.	Oğlaklı	0.1599	0.1172	0.0937	0.0802	6690.0	6090.0	0.0524	0.0458
	Uzungol	0.1352	0.1040	0.0859	0.0749	0.0662	0.0587	0.0516	0.0457
	Arca	0.0910	0.0785	0.0699	0.0639	0.0582	0.0538	0.0498	0.0456
	Ballica	0.0919	0.0790	0.0702	0.0641	0.0584	0.0540	0.0499	0.0456
	Güneşli-2	0.0940	0.0803	0.0711	0.0647	0.0588	0.0542	0.050.0	0.0456
	Çaykara	0.0976	0.0825	0.0725	0.0657	0.0596	0.0547	0.0501	0.0456
Flow	Percentile	%9	40%	42%	%0Z	%27	%0E	%98	70%

### 5.3 Annual Model Development

### 5.3.1 Parameter Selection Using Principal Component Analysis

Principal Components Analysis (PCA) is an alternative method for selecting a subset of variables for use in developing a model. It is based on an orthogonal rotation of the correlation matrix. The objective of PCA is to select a subset of variables that are important but relatively uncorrelated (McCuen, 1993).

PCA does not provide a prediction equation; this method only provides the user with the information needed to select a subset of variables. There is also some subjectivity and the selection of parameters depends on the user's personal knowledge of the system and experience (McCuen, 1993).

The data set (7 predictors and a criterion variable) is input to the program PCA for 8 models. Every model differs from each other by different specific flow values corresponding to various exceedance probabilities as indicated before. The output for 15% annual model is provided in Appendix-A.

Predictor variables which are used in model formulation are listed in Table 5.4.

The accepted parameters to be used in the multiple regression models. They are given in Table 5.5.

Table 5.4 Predictor Variables

Predictor Variables (x's)	Type of Data (Category)
X <sub>1</sub> : Perimeter (P) km	Linear measure parameter
X <sub>2</sub> : Mean Slope of the Basin (S) %	Deliaf or clane parameter
X <sub>3</sub> : Maximum Basin Relief (ΔH) m	Relief or slope parameter
X <sub>4</sub> : Perimeter/Main Stream Length (P/L)	Shape parameter
X <sub>5</sub> : Drainage Density (D <sub>d</sub> ) km <sup>-1</sup>	
X <sub>6</sub> : Drainage Frequency (D <sub>f</sub> ) km <sup>-2</sup>	Morphology parameter
X <sub>7</sub> : Mean Annual Depth of Precipitation	Hydro-Meteorological
(MAP) mm	parameter

Table 5.5 The Selected Parameters after PCA

MODEL	Basin Perimeter P (km)	Average Slope of the Basin S (%)	Maximum Basin relief ΔH (m)	Basin perimeter/main stream lenght P/L	Drainage Density D <sub>d</sub> (km <sup>-1</sup> )	Drainage Frequency D <sub>f</sub> (km <sup>-2</sup> )	Mean Annual Precipitation of the Basin MAP (mm)
Annual Model 1 (5%)		x		х		х	х
Annual Model 2 (10%)		x		x		х	х
Annual Model 3 (15%)		x		х		Х	x
Annual Model 4 (20%)		x		х		х	х
Annual Model 5 (25%)		x		x		х	х
Annual Model 6 (30%)		x		х		Х	x
Annual Model 7 (35%)		x		х		Х	х
Annual Model 8 (40%)		х		x		х	х

As seen from Table 5.5 that for each flow percentile; the selected parameters are the same. For each model, 4 parameters are selected which mean that 4 eigen values, principal components of the system, totally describe almost 95% of the variation. After selecting the number of variables; identification of dominant variables associated with each principal component are performed. For this identification the eigen-vector matrix is scanned and the values that have relatively large absolute values are selected. If the values in the eigen-vector matrix are almost similar; the correlation matrix are used to help select variables for each principal component.

Mean slope of the basin (S), P/L,  $D_f$  and mean annual precipitation of the basin (MAP) are the selected parameters. These parameters are the input variables for multiple regression analysis. Each variable is selected that each one belongs to a different category as S is a relief measure, P/L is a shape measure,  $D_f$  is a morphological measure and MAP is a hydro-meteorological measure. This categorization is essential for reducing intercorrelations.

## 5.3.2 Model Development and Discussion of Results Using Multiple Regression Analysis

The objective of multiple regression analysis (MRA) is to develop a prediction equation relating a criterion variable to predictor variables. MRA can lead to significant increases in prediction accuracy and the ability to measure the effect of each independent variable on the dependent variable (McCuen, 1993).

Totally, eight models are tested by using multiple regression analysis method. Each model differs from each other as the dependent hydrologic variable, specific runoff, corresponding to different flow percentiles. Starting from the first model to eight model; exceedance probabilities are 5%, 10%, 15%, 20%,25%, 30%, 35% and 40%.

The independent variables are the same for all models which are outputs of PCA as they are described in Chapter 5.3.

The variables for each model are input to the program MULTREG. The summary table for every model is given in Table 5.6. The output for 15% annual model is provided in Appendix-B.

Table 5.6 Multiple Regression Summary for the Annual Models

MODELS	Annual Model 1 (5%)	Annual Model 2 (10%)	Annual Model 3 (15%)	Annual Model 4 (20%)	Annual Model 5 (25%)	Annual Model 6 (30%)	Annual Model 7 (35%)	Annual Model 8 (40%)
Coefficient of S	-5.78E-03	-2.90E-03	-1.59E-03	-9.28E-04	-5.70E-04	-2.92E-04	-1.44E-04	-5.36E-05
t <sub>s</sub>	-1.07	-1.07	-1.01	-0.86	-0.71	-0.47	-0.27	-0.11
Coefficient of P/L	-4.38E-02	-2.23E-02	-1.24E-02	-7.77E-03	-4.87E-03	-2.91E-03	-1.87E-03	-1.26E-03
t <sub>P/L</sub>	-0.58	-0.59	-0.56	-0.52	-0.44	-0.33	-0.25	-0.19
Coefficient of D <sub>f</sub>	3.87E-02	-3.96E-04	-2.05E-02	-2.38E-02	-2.41E-02	-2.16E-02	-1.99E-02	-1.76E-02
t <sub>Df</sub>	0.56	0.00	-0.10	-0.17	-0.23	-0.27	-0.29	-0.29
Coefficient of MAP	5.35E-05	3.37E-05	2.54E-05	1.91E-05	1.52E-05	1.14E-05	9.10E-06	7.40E-06
t <sub>MAP</sub>	0.46	0.57	0.74	0.82	0.87	0.84	0.79	0.72
Intercept coefficient	0.37	0.21	0.13	0.09	0.06	0.04	0.03	0.02
Multiple R <sup>2</sup>	0.88	0.83	0.73	0.65	0.61	0.59	0.61	0.63
$S_e/S_y$	0.40	0.48	0.61	0.69	0.72	0.74	0.73	0.71

If Table 5.6 is examined carefully; it is seen that the signs of mean basin slope of the basin and P/L are negative for all models. The sign of mean annual precipitation is positive for all models which is physically rational. However; the sign of drainage frequency is positive for the first annual model and negative for the rest. Irrationalities for variables may occur because of intercorrelations between parameters.

Standardized partial regression coefficient ( $t_i$ ) is an indicator of the relative importance of the variable concerned.  $t_i$  values are a function of both the intercorrelations and predictor-criterion correlation coefficients (McCuen, 1993). The larger  $t_i$  value means the more important the variable. Therefore,

ti values are good indicators for evaluating the importance of the parameters since it both takes notice of intercorrelations and predictor-criterion coefficient. Coefficient of multiple determination (R<sup>2</sup>) is the fraction of the variation in the criterion variable that is explained by the regression equation (McCuen, 1993). In other words, it is the ratio of the explained variation to the total variation showing the ability of regression equation to explain the reality.

When  $t_i$  values are compared, it can be concluded that MAP and S variables are generally most important parameters. However, absolute value of  $t_i$  being more than 1 meaning that there are some intercorrelations between variables.

Furthermore, when the multiple  $R^2$  values are investigated, it is seen that the value is decreasing from the first model to the last model. Also the value  $S_e/S_y$  (the ratio of standard error of estimate  $(S_e)$  to the standard deviation  $(S_y)$ ) also increases showing that model quality decreases when the flow percentile increases. The reasons why the quality of model decreases towards 40% are the effect of baseflow on low flows which is especially significant in Eastern Black Sea Basin and the lack of other parameters which is not included in the model.

# 5.3.3 Model Development and Discussion of Results Using Stepwise Regression Analysis

The objective of stepwise regression is to develop a prediction equation relating a criterion variable to p predictor variables. Although it is a type of multiple regression analysis it differs from the commonly used multiple regression technique in that stepwise regression, in addition to calibrating a predicting equation, introduces predictor variables sequentially based on a partial-F statistic; thus stepwise regression analysis yields p prediction equations from which one must be selected as the best model. Unlike the multiple regression technique, stepwise regression usually avoids the irrational coefficients because the final model can be selected so that only predictor variables with low intercorrelation are included (McCuen, 1993).

Eight models are tested as the same as multiple regression analyses. The data set (7 predictors and a criterion variable) is input to the program STEPWISE for every model. The parameters here are selected step by step automatically according to partial F test. The most important significant variable which has the highest partial F value enters the equation first. Insertion of parameters goes on until the model becomes significant. The significance of the entire prediction model is tested according to total F test. If total F value is less than the critical value, the model is accepted to be the final model. Stepwise regression summary is given in Table 5.7. The output for 15% annual model is provided in Appendix-C.

Table 5.7 Summary Table for Annual Stepwise Models

	Annual Model 1	Annual Model 2	Annual Model 3	Annual Model 4	Annual Model 5	Annual Model 6	Annual Model 7	Annual Model 8
MODELS	(5%)	(10%)	(15%)	(20%)	(25%)	(30%)	(35%)	(40%)
Coefficient of P	-1.73E-04	-1.09E-04	-7.10E-05	7.30E-05	6.90E-05	6.20E-05		
t <sub>P</sub>	-0.31	-0.39	-0.43	0.66	0.84	0.96		
Coefficient of S	-4.00E-03	-2.10E-03	-9.80E-04				-3.70E-04	
t <sub>S</sub>	-0.74	-0.78	-0.62				-0.69	
Coefficient of P/L	-3.30E-02	-1.92E-02	-1.06E-02	-7.49E-03	-5.12E-03	-4.55E-03	-4.56E-03	-1.27E-03
t <sub>P/L</sub>	-0.44	-0.51	-0.49	-0.50	-0.46	-0.52	-0.61	-0.19
Coefficient of ΔH				-1.40E-05	-1.10E-05	-9.00E-06		
$t_{\DeltaH}$				-1.69	-1.77	-1.81		
Coefficient of Dd	-8.18E-02					1.91E-02	3.75E-02	
t <sub>Dd</sub>	-0.15					0.31	0.72	
Coefficient of D <sub>f</sub>								-1.56E-02
t <sub>Df</sub>								-0.26
Coefficient of MAP	4.90E-05	3.00E-05	2.20E-05	2.70E-05	2.30E-05	1.70E-05	8.00E-06	7.00E-06
t <sub>MAP</sub>	0.42	0.51	0.64	1.16	1.32	1.27	0.71	0.64
Intercept coefficient	0.36	0.18	0.10	0.05	0.03	0.01	0.01	0.02
Multiple R <sup>2</sup>	0.93	0.92	0.83	0.78	0.75	0.75	0.70	0.62
S <sub>e</sub> /S <sub>y</sub>	0.31	0.34	0.48	0.55	0.58	0.61	0.64	0.69

If Table 5.7 is examined carefully; it is seen that the signs of mean basin slope of the basin and P/L are negative for all models. The sign of mean annual precipitation is positive for all models. However; the signs of drainage density and basin perimeter change among models. The multiple determination of coefficients ( $R^2$ ), the standard error ratio ( $S_e/S_y$ ) and the standardized partial regression coefficients ( $t_i$ ) are the other indicators of model quality.

 $t_i$  values indicate that S, MAP and P are the variables that are most important. The absolute values of  $t_i$  for  $\Delta H$  are greater than 1 meaning that it has an intercorrelation resulting an irrationality with other predictor variables.

In addition; when the multiple  $R^2$  values are investigated, it is seen that the value is decreasing from the first model to the last model. Also the value  $S_e/S_y$  (the ratio of standard error of estimate  $(S_e)$  to the standard deviation  $(S_y)$ ) also increases showing that model quality decreases when the flow percentile increases. The same reasons discussed in Chapter 5.3.2 also explain this decrease in model quality. Nevertheless; the indicators about model quality shows that stepwise models are better than multiple regression models.

### 5.4 Seasonal Model Development

For the seasonal models; an additional parameter, mean seasonal temperature value of the basin (T), is included. Besides this, instead of using mean annual precipitation value of the basin, mean seasonal (spring) precipitation value of the basin (MSP) is used.

### 5.4.1 Parameter Selection Using Principal Component Analysis

The data set (8 predictors and a criterion variable) for each model is input to the program PCA. The output for 15% annual model is provided in Appendix-D.

Predictor variables which are used in model formulation are listed in Table 5.8.

Table 5.8 Predictor Variables

Predictor Variables (x's)	Type of Data (Category)
X <sub>1</sub> : Perimeter (P) km	Linear measure parameter
X <sub>2</sub> : Mean Slope of the Basin (S) %	
X <sub>3</sub> : Maximum Basin Relief (ΔH) m	Relief or slope parameter
X <sub>4</sub> : Perimeter/Main Stream Length (P/L)	Shape parameter
X <sub>5</sub> : Drainage Density (D <sub>d</sub> ) km <sup>-1</sup>	
X <sub>6</sub> : Drainage Frequency (D <sub>f</sub> ) km <sup>-2</sup>	Morphology parameter
X <sub>7</sub> : Mean Seasonal Temperature of the	
Basin (T) °C	Hydro-Meteorological
X <sub>8</sub> : Mean Seasonal Depth of Precipitation	parameter
(MSP) mm	

In Table 5.9, one can see the selected parameters to be used in multiple regression analysis (MRA).

Table 5.9 The Selected Parameters after PCA

				Basin			Mean Seasonal	
	Basin	Average	Maximum	p	-	Drainage	Basin	Precipitation
	Perimeter	Slope of the	Basin relief	stream lenght	Density	Frequency	Temperature T	of the Basin
MODEL	P (m)	Basin S (%)	ΔH (m)	P/L	$D_d (km^{-1})$	$D_f (km^{-2})$	(°C)	MSP (mm)
Annual Model 1 (5%)	х			х		х	х	
Annual Model 2 (10%)	х			x		х	x	
Annual Model 3 (15%)	х			x		х	x	
Annual Model 4 (20%)	х			x		х	x	
Annual Model 5 (25%)	х			х		х	x	
Annual Model 6 (30%)	х			х		х		х
Annual Model 7 (35%)	х			x		х		х
Annual Model 8 (40%)	х			х		х		Х

For each model, 4 parameters are selected according to the values of each eigen value representing a different principal component, in total describing almost 95% of the variation. After determining that there are 4 eigen values, in other words 4 variables representing the system, each variable is selected according to the same procedure applied in annual PCA.

Perimeter of the basin (P), P/L,  $D_f$  and mean seasonal temperature of the basin (T) are the selected parameters for the first five models as seen in Table 5.9. For the last three models, mean seasonal precipitation (MSP) of the basin

was selected instead of T. These parameters are the input variables for multiple regression analysis. Please note that each variable belongs to a different category meaning that selection was done considering reducing intercorrelations.

# 5.4.2 Model Development and Discussion of Results Using Multiple Regression Analysis

Totally, eight models are tried by using multiple regression analysis method. Each model differs from each other as the dependent hydrologic variable, specific runoff, corresponding to different flow percentiles. Starting from the first model to eight model; flow percentiles are 5%, 10%, 15%, 20%,25%, 30%, 35% and 40%.

The independent variables are the same for all models which are outputs of PCA as they are described in Chapter 5.6. Each variable for the models are input to the program MULTREG. The summary table for every model is given in Table 5.10. The output for 15% annual model is provided in Appendix-E.

Table 5.10 Multiple Regression Summary for the Models

	Annual	Annual	Annual	Annual	Annual	Annual	Annual	Annual
	Model 1	Model 2	Model 3	Model 4	Model 5	Model 6	Model 7	Model 8
MODELS	(5%)	(10%)	(15%)	(20%)	(25%)	(30%)	(35%)	(40%)
Coefficient of P	-1.43E-03	-7.40E-04	-4.31E-04	-2.89E-04	-2.09E-04	-7.20E-05	-3.30E-05	-1.30E-05
t <sub>P</sub>	-1.25	-1.29	-1.29	-1.27	-1.26	-0.65	-0.49	-0.24
Coefficient of P/L	-3.56E-02	-1.85E-02	-1.14E-02	-8.74E-03	-6.51E-03	-6.48E-03	-3.45E-03	-1.92E-03
t <sub>P/L</sub>	-0.23	-0.24	-0.25	-0.28	-0.29	-0.44	-0.38	-0.27
Coefficient of D <sub>f</sub>	5.34E-01	2.34E-01	1.23E-01	7.57E-02	5.02E-02	3.12E-02	1.18E-02	2.12E-03
t <sub>Df</sub>	0.34	0.29	0.27	0.24	0.22	0.20	0.13	0.03
Coefficient of T	5.59E-02	2.80E-02	1.56E-02	1.01E-02	7.21E-03			
t <sub>T</sub>	0.76	0.76	0.73	0.69	0.68			
Coefficient of MAP						7.00E-06	1.90E-05	2.50E-05
t <sub>MAP</sub>						0.07	0.31	0.51
Intercept coefficient	-0.08	0.02	0.05	0.06	0.06	0.07	0.05	0.04
Multiple R <sup>2</sup>	0.91	0.93	0.95	0.95	0.95	0.78	0.64	0.50
S <sub>e</sub> /S <sub>y</sub>	0.35	0.30	0.26	0.26	0.27	0.54	0.70	0.83

If Table 5.10 is examined carefully; it is seen that the signs of basin perimeter and P/L are negative for all models. The signs of mean seasonal precipitation and temperature and also drainage frequency are positive for all models. This is an indication of physically rationality. No ever parameter changes sign so all the models are consistent in this manner.

Furthermore, when the multiple  $R^2$  values are investigated, it is seen that the value is decreasing from the first model to the last model. Also the value  $S_e/S_y$  which indicates the standard error of estimate also increases showing that model quality decreases when the flow percentile increases. The effect of baseflow and lack of some significant parameters again influence the model

quality in spring model. Moreover, the decrease in model quality is more striking.

When the absolute values of  $t_{\rm i}$  are examined, it can be concluded that T variable which indicates the mean seasonal temperature value is important in this manner.

# 5.4.3 Model Development and Discussion of Results Using Stepwise Regression Analysis

Eight models are tested as the same as multiple regression analyses.

The data set (8 predictors and a criterion variable) is input to the program STEPWISE. Stepwise regression summary is given in Table 5.7. The output for 15% annual model is provided in Appendix-F.

Table 5.11 Summary Table for Stepwise Models

	Annual	Annual	Annual	Annual	Annual			Annual
	Model 1	Model 2	Model 3	Model 4	Model 5	Annual Model	Annual Model	Model 8
MODELS	(5%)	(10%)	(15%)	(20%)	(25%)	6 (30%)	7 (35%)	(40%)
Coefficient of P	-9.29E-04	-6.66E-04	-3.41E-04	-2.88E-04	-2.09E-04	-1.32E-04	-6.90E-05	
t <sub>P</sub>	-0.81	-1.16	-1.02	-1.27	-1.26	-1.20	-1.04	
Coefficient of P/L			-7.13E-03	-8.74E-03	-6.51E-03	-4.86E-03	-2.13E-03	
t <sub>P/L</sub>			-0.16	-0.29	-0.29	-0.33	-0.24	
Coefficient of ΔH		1.10E-05						-4.00E-06
t <sub>∆H</sub>		0.26						-0.94
Coefficient of D <sub>f</sub>				7.57E-02	5.02E-02	2.75E-02		
t <sub>Df</sub>				0.24	0.22	0.18		
Coefficient of D <sub>d</sub>	-6.28E-02	-2.79E-01	-1.08E-01					
t <sub>Dd</sub>	-0.57	-0.50	-0.34					
Coefficient of T	3.23E-02	1.87E-02	1.11E-02	1.01E-02	7.21E-03	4.46E-03	2.66E-03	2.41E-03
t <sub>T</sub>	0.44	0.50	0.52	0.69	0.68	0.63	0.62	0.70
Coefficient of MAP							2.20E-05	4.90E-05
t <sub>MAP</sub>							0.37	1.02
Intercept coefficient	0.51	0.25	0.17	0.06	0.06	0.06	0.05	0.03
Multiple R <sup>2</sup>	0.95	0.95	0.94	0.95	0.95	0.89	0.73	0.60
S <sub>e</sub> /S <sub>y</sub>	0.25	0.27	0.28	0.26	0.27	0.40	0.61	0.70

If Table 5.11 is examined carefully; it is seen that the signs of basin perimeter, P/L and drainage density are negative for all models. The signs of mean seasonal precipitation and temperature and also drainage frequency are positive for all models. Only maximum basin relief parameter changes sign showing inconsistency.

Furthermore, when the multiple  $R^2$  values are investigated, it is seen that the value is decreasing from the first model to the last model. Also the value  $S_e/S_y$  which indicates the standard error of estimate also increases showing that model quality decreases when the flow percentile increases. The same

reasons as discussed in Chapter 5.4.2 are valid for explaining the decrease in model quality. Nevertheless; the indicators about model quality shows that stepwise models are better than multiple regression models.

When the absolute values of  $t_i$  are examined, it can be concluded that T variable which indicates the mean seasonal temperature value is important in this manner.

When the absolute values of  $t_i$  are examined, it can be concluded that T variable is important as the same as MRA results.

### 5.5 Validation of Results

The results of the statistical models set up above are compared with the selected flow gauging station; 22-52. The results of drainage area ratio method are also compared. 22-52 is treated as an ungauged basin and flow values of the flow gauging stations 22-52 are derived from the sum of the flow values of 22-07 and 22-57 gauging stations upstream of 22-52.

22-52 is a station operated by DSI and its flow records are from 1979 till 2009. It is located in the Solaklı Basin and has a drainage area of 563.22 km<sup>2</sup>.

The validation results for each annual and seasonal models are given below in Table 5.12 and 5.13 respectively. In Figure 5.1 and 5.2 one can see the graphical comparisons.

In Table 5.12 and Figure 5.1, it can be concluded that multiple regression models provide better estimations compared to stepwise models. Stepwise regression model is better than multiple regression model; only for the model corresponding to 40% exceedance probability. Up to 25%, drainage area ratio method is better; but then the results of MRA and stepwise regression provide either better or same estimations. However, when Table 5.13 and Figure 5.2 is examined, models corresponding to percentiles ranging from 5% to 15% provide better estimations for stepwise models, but after 20% to 40%, either multiple regression models are better or they are equally well estimated and also the results of drainage area ratio method are almost perfect and much better than that of MRA and stepwise regression results.

When Table 5.12 is examined, the relative error values for MRA and Stepwise models are increasing towards 40% exceedance probabilities which are consistent with the  $R^2$  and  $S_e/S_y$  values. In Table 5.13, it is seen that relative errors for both methods are not increasing as much as in annual models but they could still be accepted to be consistent with the model quality indicators of MRA and Stepwise models as discussed in Chapter 5.3 and 5.4.

One of the reasons of the decrease in the model quality towards 40% is the baseflow is not considered as mentioned before. Besides, absence of some other parameters related with the basin such as; geologic/soil data, land use

and land cover data may have increased the error in the models. Intercorrelations between the parameters (especially between specific flow values some of which are derived from flow gauging stations that are present in the calibration) and lack of meteorological stations with poor quality and discontinuous data resulting poor areal rainfall values also affect the model quality. The degrees of freedom are so low that specific flow values of ungauged basins had to be estimated which caused intercorrelations which decrease model quality.

The results of the drainage area ratio method are seemed to be better than the other methods up to 30% exceedance probability for the annual models. After that, they are either equal or worse than the results of MRA and stepwise regression. However for the seasonal models the results of drainage area ratio method are much better. Because some of the values of 22-57 and 22-07, the upstream stations that are used to derive flows of 22-52, are completed from 22-52; drainage area ratio method provided better estimations. Some of the flows of 22-57 and 22-07 are also corrected as stated in Chapter 4.1.3.2 which also results better estimation. Since these corrections are present in spring season; the observed values and the values resulting from drainage area ratio method almost match for the spring model.

Table 5.12 Validation Results for Annual Models

Flow percentile (%)	Specific Discharge Values for MRA (m³/s/km²)	Discharge Values for MRA (m³/s)	Specific Discharge Values for Stepwise (m³/s/km²)	Discharge Values for Stepwise (m³/s)	Observed values of 22-52 (m <sup>3</sup> /s)	Relative error for MRA (%)	Relative error for Stepwise (%)	Discharge Values for Drainage Area Ratio Method (m³/s)	Relative error for Drainage area ratio method (%)
5	0,0753	42,4146	0,0718	40,4460	43,0000	1,3615	5,9395	42,81	0,4419
10	0,0585	32,9411	0,0553	31,1626	33,0000	0,1785	5,5679	33,53	1,6061
15	0,0465	26,1822	0,0459	25,8501	27,6000	5,1371	6,3403	26,81	2,8623
20	0,0377	21,2210	0,0363	20,4650	22,2000	4,4099	7,8152	21,44	3,4234
25	0,0300	16,8889	0,0295	16,6002	18,9000	10,6407	12,1684	17,02	9,9471
30	0,0238	13,4233	0,0224	12,6108	15,9000	15,5767	20,6868	13,98	12,0755
35	0,0201	11,3263	0,0189	10,6635	13,2000	14,1950	19,2161	11,5	12,8788
40	0,0165	9,3140	0,0180	10,1364	11,6000	19,7065	12,6170	9,73	16,1207

Table 5.13 Validation Results for Seasonal Models

Flow percentile (%)	Specific Discharge Values for MRA (m <sup>3</sup> /s/km <sup>2</sup> )	Discharge Values for MRA (m <sup>3</sup> /s)	Specific Discharge Values for Stepwise (m <sup>3</sup> /s/km <sup>2</sup> )	Discharge Values for Stepwise (m³/s)	Observed values of 22-52 (m <sup>3</sup> /s)	Relative error for MRA (%)	Relative error for Stepwise (%)	Discharge Values for Drainage Area Ratio Method (m³/s)	Relative error for Drainage area ratio method (%)
5	0,1194	28,7538	0,1145	27,5667	57,0000	49,5547	51,6374	57,3	0,5263
10	0,0937	22,5669	0,0956	23,0149	49,0000	53,9451	53,0308	47,69	2,6735
15	0,0791	19,0531	0,0782	18,8155	43,3000	55,9976	56,5463	42,99	0,7159
20	0,0698	16,7949	0,0699	16,8189	39,5000	57,4813	57,4205	39,57	0,1772
25	0,0622	14,9831	0,0622	14,9830	35,8000	58,1479	58,1482	36,25	1,2570
30	0,0560	13,4906	0,0558	13,4366	33,0000	59,1193	59,2831	33,13	0,3939
35	0,0511	12,3135	0,0514	12,3720	30,8000	60,0211	59,8312	30,29	1,6558
40	0,0464	11,1653	0,0461	11,1065	28,5000	60,8234	61,0298	27,56	3,2982

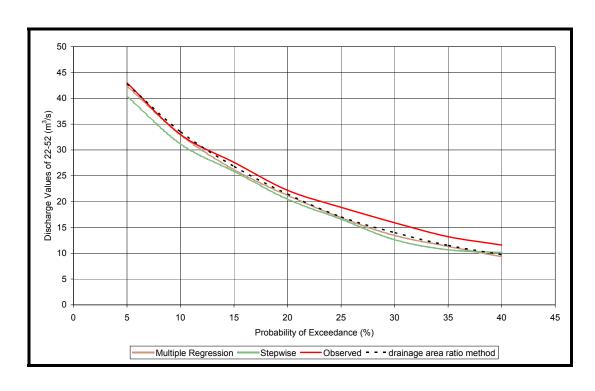


Figure 5.1 Comparison of Annual Multiple and Stepwise Models with Observed FDC and Drainage Area Ratio Method

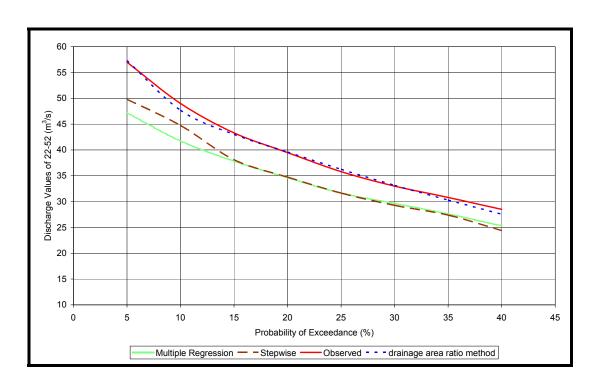


Figure 5.2 Comparison of Seasonal Multiple and Stepwise Models with Observed FDC and Drainage Area Ratio Method

### **CHAPTER 6**

### **CONCLUSIONS AND RECOMMENDATIONS**

#### 6.1 Conclusions

Estimation of specific flow values corresponding to various exceedance probabilities of the flow duration curves (5% to 40%) by statistical approach was done in this study. In order to realise this objective; flow duration curves were estimated by developing multiple regression and stepwise analyses using the specific runoff values, basin topographic characteristics, morphological and meteorological variables. Furthermore; this study examines the application of statistical models for simulation of the flow duration curve with limited data and smaller amount of gauged basins. It can be concluded that regional specific flow duration curves can be derived by using some of the variables from each category being shape, morphological, hydro-meteorological, relief/slope measures.

Seasonal specific FDCs are also derived using the same parameters for annual models in addition to mean seasonal temperature value of the median elevation of basin. Mean seasonal (spring) temperature values become significant in the models meaning that this variable affects the specific flow values for each percentile of the basins located in Solaklı and Karadere basins. Since temperature values are related with snow covered areas of each

basin, it can be concluded that in spring season amount of snowmelt affect specific runoff of the basins in Solaklı and Karadere.

The main conclusions are as follows:

Both stepwise regression analysis and MRA provide an underestimation of discharges for the seasonal and annual models; but annual model results are much better than seasonal model results.

Annual model results of multiple regression analysis corresponding to exceedance probabilities from 5% to 10% almost match with the observed values of 22-52 station which is the station used for validation and better than the results of stepwise regression model. From 10% to 20%; the results of MRA are still applicable, but from 25% to 40%; the model quality decreases significantly; but the results of MRA are still better than or same as the results of stepwise model.

In the annual models of MRA, the most sensitive and important variables are mainly MAP and S variables meaning that the meteorological and slope/relief measures are dominant for the basins concerned. These results are almost consistent with the results of stepwise regression results.

Seasonal model results of both multiple regression analysis and stepwise regression analysis did not provide good results. Stepwise regression results

are better than MRA results for the flow values corresponding to the exceedance probabilities from 5% to 15%. However, from 15% to 40% MRA results are either better than or as the same as the results of stepwise regression.

For the seasonal models of MRA, the most sensitive and important variable is the T variable which represents mean seasonal temperature value meaning that the effect of snowmelt is the most important process for the surface runoff in this region. These results are almost consistent with the results of stepwise regression results.

It can be reached a conclusion that MRA provides better estimations of flow duration curves than stepwise regression analysis for annual models and annual model estimation is better than seasonal model estimation. Although the results of drainage area ratio method provide the best results, the similarities between flows of 22-52 and other upstream stations, because of some corrections and extrapolations, must have caused perfect match between the observed values and drainage area method.

The final model parameters and related equations are only valid in Solaklı and Karadere basins. These models can also be applied in similar basins according to hydrological similarity techniques and spatial proximity criteria offered by Li et al. (2010) or the same hydrological region (Region A) offered by Yanık et al. (2005).

Since the exceedance probabilities of project discharges of small HEPPs lie between the range of 5% and 25% of the FDCs; the results of this study can be used in estimation of project discharges in ungauged basins within Solakli and Karadere basins and similar basins where small HEPPs are planned/designed.

### 6.2 Recommendations

There are also some recommendations which can be listed as follows:

- The network for streamflow gauging stations should be broadened and it should be controlled in terms of quality and quantity.
- The meteorological and climatological stations should be located also at upper elevations of the pilot areas by governmental and private organizations and the reliable data should be collected continuously in a daily basis for long term period.
- Simulation studies for the ungauged basins could be performed by using similarity indices such as spatial proximity, neighborhood indices, etc (Masih, et al., 2010).

- In order to form more accurate regressional models; land use data, soil data, geology data should also be obtained other than topographical and hydro-meteorological data.
- Relevant geostatistical techniques should be used in order to estimate
  mean daily areal values of important hydro-meteorological
  parameters such as rainfall, snow and temperature. Furthermore, the
  correction factors, such as lapse rate, used for rainfall and snow
  measurements should be established based on correlation analyses.
- Usage of snow depletion line instead of median basin elevation to transfer the representative temperature values within the snow covered area may increase the model accuracy and quality.
- Evaluation of model performance should be satisfied with more statistical methods like; Nash-Sutcliffe Efficiency (NSE) and volume balance.
- The flow duration curves should be regionalized by using appropriate statistical distribution functions with 2 or 3 parameters and using more stream flow gauging stations.

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### **APPENDIX A: PCA OUTPUT FOR 15% ANNUAL MODEL**

Data Matrix

161.500000 .550000 1816.110	44.850000 000 .045		3.610000	.640000						
179.900000 .540000	45.430000	3191.000000	3.430000	.640000						
1746.170000 .045300										
194.000000 .540000	44.940000	3263.000000	3.390000	.640000						
1703.170	000 .045	5000								
202.300000 .550000	44.750000	3294.000000	3.280000	.640000						
1676.340	000 .044	1800								
83.600000 .550000	47.960000	2373.000000	3.090000	.640000						
1659.080	000 .050	0600								
61.600000 .460000	41.790000	1845.000000	3.100000	.630000						

1580.310000 .053300

67.400000 .600000	36.910000	2105.000000	3.970000	.670000
1639.060	000 .052	2200		
74.500000 .500000	35.080000	1186.000000	3.580000	.630000
1168.390	000 .051	1500		
13.300000 .640000	25.740000	964.000000	2.860000	.410000
1169.750	000 .067	7700		
79.500000 .500000	35.490000	1418.000000	3.390000	.620000
1166.070	000 .051	1300		
89.100000 .560000	36.770000	1500.000000	4.060000	.630000
1190.070	000 .049	9700		
100.300000	40.270000	2379.000000	3.740000	.650000
1585.290	000 .042	2800		
72.400000 .560000	47.590000	2240.000000	2.850000	.640000
1693.430	000 .055	5800		
178.600000 .540000	41.560000	2755.000000	3.200000	.630000
1122.460	000 .033	3400		

134.200000 38.750000 2322.000000 4.030000 .620000 .540000

1168.430000 .031700

88.800000 36.650000 1503.000000 4.120000 .570000

.630000

1190.400000 .034000

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

\* \*

# STATISTICS FOR UNTRANSFORMED DATA

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VAR	MEAN	ST DEV	COEFF of VAR
1	111.3125000	56.0757600	.5037687
2	40.2831300	5.8082200	.1441849
3	2213.4380000	763.3208000	.3448576
4	3.4812500	.4161710	.1195464
5	.6187500	.0593155	.0958635
6	.5500000	.0453137	.0823886
7	1454.6580000	267.1718000	.1836664
8	.0471813	.0091287	.1934805

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

\* \*

## CORRELATION MATRIX

\_\_\_\_\_

var	1	2	3	4	5	6	7	8
1	1.000	.600	.884	.096	.467	205	.350	631
2	.600	1.000	.795	205	.713	370	.735	315
3	.884	.795	1.000	087	.537	165	.699	436
4	.096	205	087	1.000	.287	.174	240	552
5	.467	.713	.537	.287	1.000	553	.462	425
6	205	370	165	.174	553	1.000	115	.090
7	.350	.735	.699	240	.462	115	1.000	.145
8	631	315	436	552	425	.090	.145	1.000

Determinant of R = .0001124

Total Sphericity Test of R = I

Computed Chi square = 104.57

degrees of freedom = 28

Chi Square

Prin. Eigen- Percent Cumulative for partial

Comp. value trace percent sphericity test df

1	3.8784	48.48	48.48	104.57	28
2	1.8167	22.71	71.19	73.03	21
3	1.1134	13.92	85.11	53.94	15
4	.7370	9.21	94.32	37.83	10
5	.2770	3.46	97.78	18.98	6
6	.1155	1.44	99.23	8.84	3
7	.0498	.62	99.85	3.43	1
8	.0122	.15	100.00	.00	0

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

## EIGENVECTOR MATRIX

Var	Sta	Standardized Eigenvector (e ** 2 / lambda)						oda)
	1	2	3	4	5	6	7	8
1	.827	.230	263	.354	.227	.099	.078	046
2	.903	261	.011	066	304	015	.137	.021
3	.917	083	337	.114	.114	001	080	.077
4	.034	.868	.027	455	.168	061	.076	.026
5	.798	.145	.425	331	056	.206	075	016
6	422	.155	835	245	159	.123	013	010
7	.670	541	223	412	.109	158	033	045
8	523	774	.052	218	.228	.139	.077	.027

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

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Communalities for Eigenvector 1 to

-----

var	1	2	3	4	5	6	7	8
1	.683	.736	.805	.931	.982	.992	.998	1.000
2	.816	.884	.884	.889	.981	.981	1.000	1.000
3	.841	.848	.961	.975	.988	.988	.994	1.000
4	.001	.754	.755	.962	.990	.994	.999	1.000
5	.637	.658	.839	.948	.952	.994	1.000	1.000
6	.178	.202	.899	.959	.985	1.000	1.000	1.000
7	.448	.740	.790	.960	.972	.997	.998	1.000

8 .273 .872 .875 .922 .974 .993 .999 1.000

\*

# APPENDIX B: MULTIPLE REGRESSION ANALYSIS OUTPUT FOR 15% ANNUAL MODEL

DATA MATRIX

========

44.850000	3.610000	.550000	1816.110000	.045800
45.430000	3.430000	.540000	1746.170000	.045300
44.940000	3.390000	.540000	1703.170000	.045000
44.750000	3.280000	.550000	1676.340000	.044800
47.960000	3.090000	.550000	1659.080000	.050600
41.790000	3.100000	.460000	1580.310000	.053300
36.910000	3.970000	.600000	1639.060000	.052200
35.080000	3.580000	.500000	1168.390000	.051500
25.740000	2.860000	.640000	1169.750000	.067700
35.490000	3.390000	.500000	1166.070000	.051300
36.770000	4.060000	.560000	1190.070000	.049700
40.270000	3.740000	.540000	1585.290000	.042800
47.590000	2.850000	.560000	1693.430000	.055800
41.560000	3.200000	.540000	1122.460000	.033400
38.750000	4.030000	.540000	1168.430000	.031700
36.650000	4.120000	.630000	1190.400000	.034000

\*

# STATISTICS FOR UNTRANSFORMED DATA

\_\_\_\_\_

		Standard	Coefficient	
Var Maximum	Mean	Deviation	of Variation	Minimum
1 47.96000	40.2831300	5.8082200	.1441849	25.7400000
2 4.120000		.4161710	.1195464	2.8500000
3 .6400000	.5500000	.0453137	.0823886	.4600000
4 1816.110		267.1718000	.1836664	1122.4600000
5 .0677000	.0471813	.0091287	.1934805	.0317000

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

## CORRELATION MATRIX

ROW	1	2	3	4	5
1	1.000	205	370	.735	315
2	205	1.000	.174	240	552
3	370	.174	1.000	115	.090
4	.735	240	115	1.000	.145
5	315	552	.090	.145	1.000

.3429309 = Determinant of intercorrelation matrix

\*\*\*\*\*

Var	b	t	R	R**2	t*R
1	0015946	-1.01458	31517	.09933	.31977
2	0123724	56405	55229	.30502	.31152
3	0205091	10181	.09009	.00812	00917
4	.0000254	.74331	.14532	.02112	.10802

.1288239 = Intercept

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

# ANALYSIS OF RESIDUALS

OBS	PREDICTED	OBSERVED	RESIDUAL	REL ERROR
NO.	YP	Y	e = YP - Y	e / Y
1	.0474858	.0458000	.0016858	.03681
2	.0472168	.0453000	.0019168	.04231
3	.0474010	.0450000	.0024010	.05336
4	.0481784	.0448000	.0033784	.07541
5	.0449722	.0506000	0056278	11122
6	.0545324	.0533000	.0012324	.02312
7	.0501708	.0522000	0020292	03887
8	.0480114	.0515000	0034886	06774
9	.0689763	.0677000	.0012763	.01885
10	.0496495	.0513000	0016505	03217
11	.0386979	.0497000	0110021	22137
12	.0475236	.0428000	.0047236	.11036
13	.0491989	.0558000	0066011	11830
14	.0403932	.0334000	.0069932	.20938
15	.0357723	.0317000	.0040723	.12847
16	.0367196	.0340000	.0027196	.07999

\*

# GOODNESS-OF-FIT STATISTICS \_\_\_\_\_ .7301207 = MULTIPLE R SQUARE .8544710 = MULTIPLE R .0055378 = STANDARD ERROR OF ESTIMATE (Se) .0091287 = STANDARD DEVIATION (Sy) .6066442 = Se/Sy.0854835 = MEAN RELATIVE ERROR .0616848 = STANDARD DEVIATION OF RELATIVE ERRORS \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 7.440 = F FOR ANALYSIS OF VARIANCE ON R

N.D.F.1 = 4. N.D.F.2 = 11.

# DISTRIBUTION OF RESIDUALS FOR NORMALITY CHECK

# CELL STANDARDIZED

VARIATE	FREQUENCY
1	.0
200000E+01	
2	1.0
150000E+01	
3	2.0
100000E+01	
4	1.0
500000E+00	
5	2.0
.000000E+00	
6	6.0
.500000E+00	
7	3.0
.100000E+01	
8	1.0
.150000E+01	
9	.0
.200000E+01	
10	.0

# APPENDIX C: STEPWISE REGRESSION ANALYSIS OUTPUT FOR 15% ANNUAL MODEL

DATA MATRIX

-----

161.50000 1816.11000		3077.00000	3.61000	.64000	.55000
179.90000 1746.17000	45.43000 .04530	3191.00000	3.43000	.64000	.54000
194.00000 1703.17000		3263.00000	3.39000	.64000	.54000
202.30000 1676.34000	44.75000 .04480	3294.00000	3.28000	.64000	.55000
83.60000 1659.08000		2373.00000	3.09000	.64000	.55000
61.60000 1580.31000		1845.00000	3.10000	.63000	.46000
67.40000 1639.06000	36.91000 .05220	2105.00000	3.97000	.67000	.60000
74.50000 1168.39000	35.08000 .05150	1186.00000	3.58000	.63000	.50000

13.30000 1169.75000	25.74000 .06770	964.00000	2.86000	.41000	.64000
79.50000 1166.07000	35.49000 .05130	1418.00000	3.39000	.62000	.50000
89.10000 1190.07000		1500.00000	4.06000	.63000	.56000
100.30000 1585.29000		2379.00000	3.74000	.65000	.54000
72.40000 1693.43000		2240.00000	2.85000	.64000	.56000
178.60000 1122.46000		2755.00000	3.20000	.63000	.54000
134.20000 1168.43000	38.75000 .03170	2322.00000	4.03000	.62000	.54000
88.80000 1190.40000		1503.00000	4.12000	.57000	.63000
******	*****	*****	******	******	*****

# CHARACTERISTICS OF DATA

\_\_\_\_\_\_

		Standard Coe	eff. of	
Var Maximum	Mean	deviatio	n variation	Minimum
1 202.3000	111.312500 00	56.075760	.503769	13.300000
2 47.96000	40.283130	5.808220	.144185	25.740000
3 3294.000		763.320800	.344858	964.000000
4 4.120000	3.481250	.416171	.119546	2.850000
5 .670000	.618750	.059316	.095863	.410000
6 .640000	.550000	.045314	.082389	.460000
7 1816.110		267.171800	.183666	1122.460000
8 .067700	.047181	.009129	.193480	.031700
*****	*****	*****	******	*****

## CORRELATION MATRIX

\_\_\_\_\_

Var	1	2	3	4	5	6	7	8
1	1.000	.600	.884	.096	.467	205	.350	631
2	.600	1.000	.795	205	.713	370	.735	315
3	.884	.795	1.000	087	.537	165	.699	436
4	.096	205	087	1.000	.287	.174	240	552
5	.467	.713	.537	.287	1.000	553	.462	425
6	205	370	165	.174	553	1.000	115	.090
7	.350	.735	.699	240	.462	115	1.000	.145
8	631	315	436	552	425	.090	.145	1.000

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\_\_\_\_\_

========

Step number = 1 Enter predictor variable 1

\_\_\_\_\_\_

## STATISTICAL CHARACTERISTICS

# FOR VARIABLE SELECTION

	Partial R	Partial F
Var	to enter	to enter
1	6308	9.254
2	3152	1.544
3	4360	3.286
4	5523	6.145
5	4253	3.092
6	.0901	.115
7	.1453	.302

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

1.0000 = Determinant of Intercorrelation Matrix

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

ERROR ANALYSIS

\_\_\_\_\_\_ Obs. Predicted Measured Error Relative ΥP No. Y e = YP - Yerror (e/Y) 1 .042027 .045800 -.003773 -.0824 2 .045300 .040138 -.005162 -.1140 -.1402 3 .038690 .045000 -.006310 4 .037837 .044800 -.006963 -.1554 5 .050027 -.000573 .050600 -.0113 6 .052286 .053300 -.001014 -.0190 7 -.0098 .051691 .052200 -.000509 -.0105 8 .050962 .051500 -.000538 9 .057246 .067700 -.010454 -.1544 -.000852 10 .050448 .051300 -.0166 11 .049462 .049700 -.000238 -.0048 12 .048312 .042800 .005512 .1288 13 .051177 .055800 -.004623 -.0828 14 .040271 .033400 .006871 .2057 15 .044831 .031700 .013131 .4142 .049493 .034000 .015493 .4557 16

\*

## GOODNESS-OF-FIT STATISTICS

\_\_\_\_\_ .3979 = Increase in R\*\*2 Due to Variable Added .3979 = Multiple R\*\*2.6308 = Multiple R .0073318 = Standard error of estimate (Se) .0091287 = Standard deviation of Y (Sy) .8031584 = Se/Sy.1253484 = Mean of Absolute Relative Errors .1371194 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 9.254 = Total F for the Analysis of Variance on R df 1 = 1. df 2 = 14.9.254 = Partial F to Enter df 1 = 1 df 2 = 14.

145

Var Se(bi)/bi	b	t	r	r**2	t*r	Se(bi)
1 .32873	000103	6308	6308	.3979	.3979	.0000
	.058612 =	Intercept				
=======	=======================================		=======	======	======	======
=======	=======================================	=======	======	=======	======	======
Step num	ber = 2	Enter pi	redictor v	ariable 4		
*****	*****	*****	*****	*****	*****	*****
******	* * *					

## STATISTICAL CHARACTERISTICS

# FOR VARIABLE SELECTION

	Partial R	Partial F
Var	to enter	to enter
2	.1017	.136
3	.3357	1.651
4	6365	8.853
5	1908	.491
6	0520	.035
7	.5038	4.422

.9907 = Determinant of Intercorrelation Matrix

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

ERROR ANALYSIS

=====	:========	========	=========	========
Obs.	Predicted	Measured	Error	Relative
No.	YP	Y	e = YP - Y	error (e/Y)
1	.041016	.045800	004784	1044
2	.041229	.045300	004071	0899
3	.040326	.045000	004674	1039
4	.040735	.044800	004065	0907
5	.054070	.050600	.003470	.0686
6	.056049	.053300	.002749	.0516
7	.046030	.052200	006170	1182
8	.049601	.051500	001899	0369
9	.063246	.067700	004454	0658
10	.051194	.051300	000106	0021
11	.042991	.049700	006709	1350
12	.045410	.042800	.002610	.0610
13	.057745	.055800	.001945	.0349
14	.043855	.033400	.010455	.3130
15	.039037	.031700	.007337	.2314
16	.042366	.034000	.008366	.2461

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

## GOODNESS-OF-FIT STATISTICS

\_\_\_\_\_ .2439 = Increase in R\*\*2 Due to Variable Added .6418 = Multiple R\*\*2 .8011 = Multiple R .0058684 = Standard error of estimate (Se) .0091287 = Standard deviation of Y (Sy) .6428574 = Se/Sy.1095873 = Mean of Absolute Relative Errors .0849105 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 11.648 = Total F for the Analysis of Variance on R df 1 = 2. df 2 = 13.8.852 = Partial F to Enter df 1 = 1 df 2 = 13.

Var Se(bi)/bi	b	t	r	r**2	t*r	Se(bi)
1 .28600	000095	5831	6308	.3979	.3678	.0000
4 .33610	010883	4962	5523	.3050	.2740	.0037
.095634 = Intercept						
=======================================						
=======		=======	=======	=======	=======	======
========						
Step numk	per = 3	Enter p	redictor va	ariable 7		
**************************************						

## STATISTICAL CHARACTERISTICS

# FOR VARIABLE SELECTION

rtial F	Partial R	
enter	to enter	Var
.272	1488	2
.241	.1404	3
.006	0216	5
.118	.0987	6
2.716	.4296	7

.7942 = Determinant of Intercorrelation Matrix

ERROR ANALYSIS

\_\_\_\_\_\_ Obs. Predicted Measured Error Relative No. ΥP Y e = YP - Yerror (e/Y) 1 .043903 .045800 -.001897 -.0414 2 .045300 -.0554 .042792 -.002508 3 .045000 -.003851 -.0856 .041149 4 .040957 .044800 -.003843 -.0858 5 .005282 .055882 .050600 .1044 6 .057493 .053300 .004193 .0787 7 .049465 .052200 -.002735 -.0524 8 .047612 .051500 -.003888 -.0755 9 .061094 .067700 -.006606 -.0976 -.0494 10 .048764 .051300 -.002536 -.007906 .041794 .049700 -.1591 11 12 .047337 .042800 .004537 .1060 13 .059673 .055800 .003873 .0694 14 .038921 .033400 .005521 .1653 15 .036781 .031700 .005081 .1603 .041283 .034000 .007283 .2142 16

\*

## GOODNESS-OF-FIT STATISTICS

\_\_\_\_\_ .0661 = Increase in R\*\*2 Due to Variable Added .7079 = Multiple R\*\*2.8414 = Multiple R .0055156 = Standard error of estimate (Se) .0091287 = Standard deviation of Y (Sy) .6042079 = Se/Sy.1000237 = Mean of Absolute Relative Errors .0497575 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 9.696 = Total F for the Analysis of Variance on R df 1 = 3. df 2 = 12.2.716 = Partial F to Enter df 1 = 1 df 2 = 12.

Var	b	t	r	r**2	t*r	Se(bi)
Se(bi)/b	L					
1 .24582	000113	6913	6308	.3979	.4361	.0000
4 .39349	009141	4167	5523	.3050	.2302	.0036
7 .60675	.000010	.2872	.1453	.0211	.0417	.0000
	.077256 = 3	Intercept				
		-				
=======	========	======	=======	=======	========	=====
======	===					
=						

Step number = 4 Enter predictor variable 2

## STATISTICAL CHARACTERISTICS

# FOR VARIABLE SELECTION

	Partial R	Partial F
Var	to enter	to enter
2	6487	7.994
3	5491	4.748
5	2880	.995
6	.1035	.119

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

\*\*\*\*\*

.2508 = Determinant of Intercorrelation Matrix

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

\*\*\*\*\*

## ERROR ANALYSIS

Obs.	Predicted	Measured	Error	Relative		
No.	YP	Y	e = YP - Y	error (e/Y)		
1	.045681	.045800	000119	0026		
2	.044201	.045300	001099	0243		
3	.043173	.045000	001827	0406		
4	.043358	.044800	001442	0322		

5	.050235	.050600	000365	0072
6	.056012	.053300	.002712	.0509
7	.052415	.052200	.000215	.0041
8	.047583	.051500	003917	0761
9	.068750	.067700	.001050	.0155
10	.048798	.051300	002502	0488
11	.040262	.049700	009438	1899
12	.048070	.042800	.005270	.1231
13	.054691	.055800	001109	0199
14	.036920	.033400	.003520	.1054
15	.034983	.031700	.003283	.1036
16	.039770	.034000	.005770	.1697

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

## GOODNESS-OF-FIT STATISTICS

-----

.1229 = Increase in R\*\*2 Due to Variable Added

.8309 = Multiple R\*\*2

.9115 = Multiple R

.0043841 = Standard error of estimate (Se)

.0091287 = Standard deviation of Y (Sy)

.4802573 = Se/Sy

.0633611 = Mean of Absolute Relative Errors

.0590819 = Std. dev. of Absolute Relative Errors

13.509 = Total F for the Analysis of Variance on R df 1 = 4. df 2 = 11.

7.994 = Partial F to Enter

df 1 = 1 df 2 = 11.

.099817 = Intercept

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.35369

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========

Step number = 5 Enter predictor variable 3					
**************************************					
STATISTICAL CHARACTERISTICS					
FOR VARIABLE SELECTION					
Partial R Partial F					
Var to enter to enter					
35420 4.160					
5 .2958 .959					
61968 .403					
**************************************					
.0102 = Determinant of Intercorrelation Matrix					
**************************************					

ERROR ANALYSIS

\_\_\_\_\_\_\_ Obs. Predicted Measured Error Relative No. ΥP Y e = YP - Yerror (e/Y) 1 .046327 .045800 .000527 .0115 -.0074 2 .045300 .044963 -.000337 -.0193 3 .044132 .045000 -.000868 4 .044672 .044800 -.000128 -.0029 5 .049092 -.001508 -.0298 .050600 6 .057013 .053300 .003713 .0697 7 .049874 .052200 -.002326 -.0446 8 .051517 .051500 .000017 .0003 9 .067328 .067700 -.000372 -.0055 -.000754 10 .050546 .051300 -.0147 -.007779 11 .041921 .049700 -.1565 12 .046064 .042800 .003264 .0763 13 .054589 .055800 -.001211 -.0217 14 .034062 .033400 .000662 .0198 15 .031528 .031700 -.000172 -.0054 .007271 .041271 .034000 .2139 16

\*

## GOODNESS-OF-FIT STATISTICS

\_\_\_\_\_ .0497 = Increase in R\*\*2 Due to Variable Added .8806 = Multiple R\*\*2 .9384 = Multiple R.0038640 = Standard error of estimate (Se) .0091287 = Standard deviation of Y (Sy) .4232851 = Se/Sy.0437009 = Mean of Absolute Relative Errors .0605128 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 14.744 = Total F for the Analysis of Variance on R df 1 = 5. df 2 = 10.4.160 = Partial F to Enter df 1 = 1 df 2 = 10.

Var	b	t	r	r**2	t*r	Se(bi)
Se(bi)/bi						
1	.000056	.3445	6308	.3979	2172	.0001
1.18321	.000036	.3443	0306	. 39 19	2173	.0001
4	011760	5361	5523	.3050	.2961	.0026
.22288						
7	.000036	1.0565	.1453	.0211	.1535	.0000
.24836						
2	000835	5313	3152	.0993	.1675	.0003
.37586						
3	000013	-1.1028	4360	.1901	.4808	.0000
.49028						

.092197 = Intercept

=======

Step number = 6 Enter predictor variable 5

## STATISTICAL CHARACTERISTICS

# FOR VARIABLE SELECTION

	Par	tial R	Par	tial F
Var	to	enter	to	enter
5		.2729		.724
6		0108		.001

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

\*\*\*\*\*\*

.0029 = Determinant of Intercorrelation Matrix

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

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## ERROR ANALYSIS

Obs.	Predicted	Measured	Error	Relative		
No.	YP	Y	e = YP - Y	error (e/Y)		
1	.045760	.045800	000040	0009		
2	.044602	.045300	000698	0154		
3	.043987	.045000	001013	0225		
4	.044774	.044800	000026	0006		
5	.048472	.050600	002128	0421		
6	.057469	.053300	.004169	.0782		

7	.051309	.052200	000891	0171
8	.052541	.051500	.001041	.0202
9	.066163	.067700	001537	0227
10	.051672	.051300	.000372	.0072
11	.041810	.049700	007890	1587
12	.046568	.042800	.003768	.0880
13	.054425	.055800	001375	0246
14	.034795	.033400	.001395	.0418
15	.031100	.031700	000600	0189
16	.039454	.034000	.005454	.1604

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

\*\*\*\*\*\*

#### GOODNESS-OF-FIT STATISTICS

\_\_\_\_\_

.0089 = Increase in R\*\*2 Due to Variable Added

.8894 = Multiple R\*\*2

.9431 = Multiple R

.0039185 = Standard error of estimate (Se)

.0091287 = Standard deviation of Y (Sy)

.4292519 = Se/Sy

.0449630 = Mean of Absolute Relative Errors

.0509564 = Std. dev. of Absolute Relative Errors

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

12.068 = Total F for the Analysis of Variance on R  $df \ 1 = 6. \qquad df \ 2 = 9.$ 

.724 = Partial F to Enter

df 1 = 1 df 2 = 9.

**************************************									
Var Se(bi)/bi	b	t	r	r**2	? t*r	Se(bi)			
1 1.28634	.000052	.3220	6308	.3979	2032	.0001			
4 .24939	013528	6167	5523	.3050	.3406	.0034			
7 .25492	.000036	1.0451	.1453	.0211	.1519	.0000			
2 .40439	001099	6993	3152	.0993	.2204	.0004			
3 .52961	000012	-1.0436	4360	.1901	.4550	.0000			
5 1.17502	.027245	.1770	4253	.1809	0753	.0320			

.091535 = Intercept

Step r	numbe	r =	7	Enter	predic	tor	varia	able	6				
****** ****			****	****	****	****	****	****	* * * *	****	***	****	* * * *
STATIS	STICA	L CHA	RACTE	RISTIC	S								
FOR	VARI.	ABLE	SELECT	TION									
					_								
	Part	ial R	. Par	rtial	F								
Var	to	enter	to	ente	r								
					_								
6		.2489		.52	8								
****** ****			****	****	*****	****	****	****	****	****	***	****	* * * *
.001	11 = 1	Deter	minant	of I	ntercor	rela	ation	Matr	ix				
*****	****	****	****	****	*****	***	* * * * * *	****	****	****	***	****	****
*****	****	*											

ERROR ANALYSIS

\_\_\_\_\_\_

Obs. Predicted Measured Error Relative No. ΥP Y e = YP - Yerror (e/Y) 1 .045328 .045800 -.000472 -.0103 2 .045300 .044342 -.000958 -.0212 3 .043952 .045000 -.001048 -.0233 .0120 4 .045337 .044800 .000537 5 .048612 .050600 -.001988 -.0393 .053300 6 .055743 .002443 .0458 7 .052438 .052200 .000238 .0046 8 .052528 .051500 .001028 .0200 9 .066230 .067700 -.001470 -.0217 10 .051460 .051300 .000160 .0031

.049700

.042800

.055800

.033400

.031700

.034000

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

-.007622

.003211

-.000403

.001987

-.001814

.006171

-.1534

.0750

-.0072

.0595

-.0572

.1815

\*\*\*\*\*\*

11

12

13

14

15

16

.042078

.046011

.055397

.035387

.029886

.040171

#### GOODNESS-OF-FIT STATISTICS

\_\_\_\_\_ .0068 = Increase in R\*\*2 Due to Variable Added .8963 = Multiple R\*\*2 .9467 = Multiple R.0040255 = Standard error of estimate (Se) .0091287 = Standard deviation of Y (Sy) .4409702 = Se/Sy.0459407 = Mean of Absolute Relative Errors .0522630 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 9.877 = Total F for the Analysis of Variance on R df 1 = 7. df 2 = 8..528 = Partial F to Enter df 1 = 1 df 2 = 8.

Var Se(bi)/bi	b	t	r	r**2	t*r	Se(bi)
1 1.02882	.000072	.4453	6308	.3979	2809	.0001
4 .28649	015568	7098	5523	.3050	.3920	.0045
7 .25840	.000036	1.0639	.1453	.0211	.1546	.0000
2 .40143	001151	7326	3152	.0993	.2309	.0005
3 .50598	000014	-1.2025	4360	.1901	.5243	.0000
5 .90782	.049502	.3216	4253	.1809	1368	.0449
6 1.37598	.027307	.1355	.0901	.0081	.0122	.0376

.073000 = Intercept

### **APPENDIX D: PCA OUTPUT FOR 15% SEASONAL MODEL**

Data Matrix

-----

161.500000 44.850000 3077.000000 .550000 4.100000 388.600000 .072500	3.610000	.640000
179.900000 45.430000 3191.000000 .540000 4.800000 373.600000 .071100	3.430000	.640000
194.000000 44.940000 3263.000000 .540000 5.200000 364.400000 .070200	3.390000	.640000
202.300000 44.750000 3294.000000 .550000 5.500000 358.700000 .069900	3.280000	.640000
83.600000 47.960000 2373.000000 .550000 2.800000 445.200000 .085900	3.090000	.640000
61.600000 41.790000 1845.000000 .460000 3.500000 424.100000 .093700	3.100000	.630000
67.400000 36.910000 2105.000000 .600000 3.000000 439.800000 .090300	3.970000	.670000
74.500000 35.080000 1186.000000 .500000	3.580000	.630000
3.300000 300.400000 .088300		
13.300000 25.740000 964.000000 .640000 3.300000 300.800000 .140900	2.860000	.410000

79.500000 35.490000 1418.000000 .500000 3.300000 299.800000 .087800	3.390000	.620000
89.100000 36.770000 1500.000000 .560000 3.000000 306.000000 .083300		.630000
100.300000 40.270000 2379.000000 .540000 3.400000 425.400000 .072300	3.740000	.650000
72.400000 47.590000 2240.000000 .560000 2.500000 454.400000 .084700	2.850000	.640000
178.600000 41.560000 2755.000000 .540000 4.000000 288.600000 .064300	3.200000	.630000
134.200000 38.750000 2322.000000 .540000 3.300000 300.400000 .061000	4.030000	.620000
88.800000 36.650000 1503.000000 .570000 3.000000 306.100000 .070600	4.120000	.630000

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\* \*

#### STATISTICS FOR UNTRANSFORMED DATA

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VAR	MEAN	ST DEV	COEFF of VAR
1	111.3125000	56.0757600	.5037687
2	40.2831300	5.8082200	.1441849
3	2213.4380000	763.3208000	.3448576
4	3.4812500	.4161710	.1195464
5	.6225000	.0579080	.0930248
6	.5462500	.0404763	.0740985
7	3.6250000	.8698660	.2399630
8	361.0188000	61.6895200	.1708762
9	.0816750	.0186974	.2289239

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

\* \*

#### CORRELATION MATRIX

\_\_\_\_\_

var 9	1	2	3	4	!	5	6	7	8
1 .762	1.000	.600	.884	.096	.450	190	.831	090	-
2.638	.600	1.000	.795	205	.687	353	.326	.601	-
3 .653	.884	.795	1.000	087	.486	093	.724	.355	-
4.453	.096	205	087	1.000	.400	.043	119	261	-
5 .806	.450	.687	.486	.400	1.000	511	.106	.394	-
6 .402	190	353	093	.043	51	1 1.0	001	370	33
7 .352	.831	.326	.724	119	.106	137	1.000	120	-
8 .014	090	.601	.355	261	.394	033	120	1.000	-
9 1.000	762	638	653	453	80	6.4	023	520	)14

Determinant of R = .0000002

Total Sphericity Test of R = I

Computed Chi square = 172.81

degrees of freedom = 36

\*

				Chi Square	
Prin.	Eigen-	Percent	Cumulative	for partial	
Comp.	value	trace	percent	sphericity test	df
1	4.3084	47.87	47.87	172.81	36
2	1.6645	18.49	66.37	133.00	28
3	1.6228	18.03	84.40	113.85	21
4	.9513	10.57	94.97	84.49	15
5	.3105	3.45	98.42	49.93	10
6	.0795	.88	99.30	26.42	6
7	.0426	.47	99.77	18.57	3
8	.0196	.22	99.99	12.04	1
9	.0008	.01	100.00	01	0

\*

#### EIGENVECTOR MATRIX

===========

Var	Standardized Eigenvector (e ** 2 / lambda)									
	1	2	3	4	5	6	7	8	9	
1	.875	.213	.427	025	.068	.014	006	.038	022	
2	.855	426	161	023	.191	022	.156	009	.003	
3	.900	190	.307	210	006	084	065	.071	.014	
4	.109	.801	384	376	210	091	.068	.018	000	
5	.772	.111	583	027	062	.214	024	.023	.003	
6	418	052	.367	814	.127	.089	.005	022	.001	
7	.620	.078	.699	.118	316	.053	.037	058	.005	
8	.296	768	409	292	251	059	040	028	009	
9	865	385	.206	.041	206	.060	.077	.087	002	

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

Communalities for Eigenvector 1 to

-----

var 9	1	2	3	4	5	6	7	8
1 1.000	.765	.810	.993	.993	.998	.998	.998	.999
2 1.000	.730	.912	.938	.939	.975	.976	1.000	1.000
3 1.000	.809	.845	.939	.983	.983	.990	.995	1.000
4 1.000	.012	.654	.801	.943	.987	.995	1.000	1.000
5 1.000	.596	.609	.948	.949	.953	.999	.999	1.000
6 1.000	.175	.178	.312	.975	.992	.999	1.000	1.000
7 1.000	.385	.391	.879	.893	.992	.995	.997	1.000
8 1.000	.088	.678	.846	.931	.994	.998	.999	1.000
9 1.000	.748	.897	.939	.941	.983	.987	.992	1.000

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

### APPENDIX E: MULTIPLE REGRESSION ANALYSIS OUTPUT FOR 15% SEASONAL MODEL

DATA MATRIX

========

161.500000	3.610000	.550000	4.100000	.072500
179.900000	3.430000	.540000	4.800000	.071100
194.000000	3.390000	.540000	5.200000	.070200
202.300000	3.280000	.550000	5.500000	.069900
83.600000	3.090000	.550000	2.800000	.085900
61.600000	3.100000	.460000	3.500000	.093700
67.400000	3.970000	.600000	3.000000	.090300
74.500000	3.580000	.500000	3.300000	.088300
13.300000	2.860000	.640000	3.300000	.140900
79.500000	3.390000	.500000	3.300000	.087800
89.100000	4.060000	.560000	3.000000	.083300
100.300000	3.740000	.540000	3.400000	.072300
72.400000	2.850000	.560000	2.500000	.084700
178.600000	3.200000	.540000	4.000000	.064300
134.200000	4.030000	.540000	3.300000	.061000
88.800000	4.120000	.570000	3.000000	.070600

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

#### STATISTICS FOR UNTRANSFORMED DATA

-----

		Standard	Coefficient	
Var Maximum	Mean	Deviation	of Variation	Minimum
1 202.3000	111.3125000 000	56.0757600	.5037687	13.3000000
2 4.1200000	3.4812500	.4161710	.1195464	2.8500000
3 .6400000	.5462500	.0404763	.0740985	.4600000
4 5.500000	3.6250000	.8698660	.2399630	2.5000000
5 .1409000	.0816750	.0186974	.2289239	.0610000

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

#### CORRELATION MATRIX

ROW	1	2	3	4	5
1	1.000	.096	190	.831	762
2	.096	1.000	.043	119	453
3	190	.043	1.000	137	.402
4	.831	119	137	1.000	352
5	762	453	.402	352	1.000

.2552212 = Determinant of intercorrelation matrix

\*\*\*\*\*

Var	b	t	R	R**2	t*R
1	0004305	-1.29126	76217	.58091	.98416
2	0113810	25332	45297	.20518	.11475
3	.1232865	.26690	.40182	.16146	.10725
4	.0156314	.72723	35206	.12395	25603

.0452111 = Intercept

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

#### ANALYSIS OF RESIDUALS

OBS	PREDICTED	OBSERVED	RESIDUAL	REL ERROR
NO.	YP	Y	e = YP - Y	e / Y
1	.0664889	.0725000	0060111	08291
2	.0703245	.0711000	0007755	01091
3	.0709616	.0702000	.0007616	.01085
4	.0745623	.0699000	.0046623	.06670
5	.0856258	.0859000	0002742	00319
6	.0948301	.0937000	.0011301	.01206
7	.0918760	.0903000	.0015760	.01745
8	.0856184	.0883000	0026816	03037
9	.1374222	.1409000	0034778	02468
10	.0856281	.0878000	0021719	02474
11	.0765774	.0833000	0067226	08070
12	.0791840	.0723000	.0068840	.09521
13	.0897227	.0847000	.0050227	.05930
14	.0609968	.0643000	0033032	05137
15	.0597248	.0610000	0012752	02090
16	.0772565	.0706000	.0066565	.09429

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

# GOODNESS-OF-FIT STATISTICS \_\_\_\_\_ .9501231 = MULTIPLE R SQUARE .9747426 = MULTIPLE R .0048762 = STANDARD ERROR OF ESTIMATE (Se) .0186974 = STANDARD DEVIATION (Sy) .2607949 = Se/Sy.0428526 = MEAN RELATIVE ERROR .0324993 = STANDARD DEVIATION OF RELATIVE ERRORS \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\*

52.386 = F FOR ANALYSIS OF VARIANCE ON R  $\text{N.D.F.1} \ = \ 4. \quad \text{N.D.F.2} \ = \ 11.$ 

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

#### DISTRIBUTION OF RESIDUALS FOR NORMALITY CHECK

#### CELL STANDARDIZED

VARIATE	FREQUENCY
1	.0
200000E+01	
2	.0
150000E+01	
3	2.0
100000E+01	
4	3.0
500000E+00	
5	4.0
.000000E+00	
6	3.0
.500000E+00	
7	1.0
.100000E+01	
8	3.0
.150000E+01	
9	.0
.200000E+01	
10	.0

## APPENDIX F: STEPWISE REGRESSION ANALYSIS OUTPUT FOR 15% SEASONAL MODEL

DATA MATRIX

-----

161.50000 44.85000 3077.00000 4.10000 388.60000 .07250	3.61000	.64000	.55000
179.90000 45.43000 3191.00000 4.80000 373.60000 .07110	3.43000	.64000	.54000
194.00000 44.94000 3263.00000 5.20000 364.40000 .07020	3.39000	.64000	.54000
202.30000 44.75000 3294.00000 5.50000 358.70000 .06990	3.28000	.64000	.55000
83.60000 47.96000 2373.00000 2.80000 445.20000 .08590	3.09000	.64000	.55000
61.60000 41.79000 1845.00000 3.50000 424.10000 .09370	3.10000	.63000	.46000
67.40000 36.91000 2105.00000 3.00000 439.80000 .09030	3.97000	.67000	.60000
74.50000 35.08000 1186.00000 3.30000 300.40000 .08830	3.58000	.63000	.50000

13.30000 25.74000 964.00000 3.30000 300.80000 .14090	2.86000	.41000	.64000
79.50000 35.49000 1418.00000 3.30000 299.80000 .08780	3.39000	.62000	.50000
89.10000 36.77000 1500.00000 3.00000 306.00000 .08330	4.06000	.63000	.56000
100.30000 40.27000 2379.00000 3.40000 425.40000 .07230	3.74000	.65000	.54000
72.40000 47.59000 2240.00000 2.50000 454.40000 .08470	2.85000	.64000	.56000
178.60000 41.56000 2755.00000 4.00000 288.60000 .06430	3.20000	.63000	.54000
134.20000 38.75000 2322.00000 3.30000 300.40000 .06100	4.03000	.62000	.54000
88.80000 36.65000 1503.00000 3.00000 306.10000 .07060	4.12000	.63000	.57000
*********	*****	*****	*****

#### CHARACTERISTICS OF DATA

\_\_\_\_\_\_

		Standard Coeff	. of	
Var Maximum	Mean	deviation	variation	Minimum
1 202.30000		56.075760	.503769	13.300000
2 47.960000	40.283130	5.808220	.144185	25.740000
3 3294.0000		763.320800	.344858	964.000000
4 4.120000	3.481250	.416171	.119546	2.850000
5 .670000	.622500	.057908	.093025	.410000
6 .640000	.546250	.040476	.074099	.460000
7 5.500000	3.625000	.869866	.239963	2.500000
8 454.40000		61.689520	.170876	288.600000
9 .140900	.081675	.018697	. 228924	.061000
*****	*****	*****	*****	*****

#### CORRELATION MATRIX

-----

Var	1	2	3	4	5	6	7	8	9
1	1.000	.600	.884	.096	.450	190	.831	090	762
2	.600	1.000	.795	205	.687	353	.326	.601	638
3	.884	.795	1.000	087	.486	093	.724	.355	653
4	.096	205	087	1.000	.400	.043	119	261	453
5	.450	.687	.486	.400	1.000	511	.106	.394	806
6	190	353	093	.043	511	1.000	137	033	.402
7	.831	.326	.724	119	.106	137	1.000	120	352
8	090	.601	.355	261	.394	033	120	1.000	014
9	762	638	653	453	806	.402	352	014	1.000

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Step number = 1 Enter predictor variable 5

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

#### STATISTICAL CHARACTERISTICS

#### FOR VARIABLE SELECTION

	Partial R	Partial F
Var	to enter	to enter
1	7622	19.405
2	6383	9.626
3	6531	10.412
4	4530	3.614
5	8056	25.890
6	.4018	2.696
7	3521	1.981
8	0137	.003

1.0000 = Determinant of Intercorrelation Matrix

ERROR ANALYSIS

\_\_\_\_\_\_ Obs. Predicted Measured Error Relative ΥP No. Y e = YP - Yerror (e/Y) 1 .077123 .072500 .004623 .0638 2 .071100 .077123 .006023 .0847 3 .077123 .070200 .006923 .0986 4 .077123 .069900 .007223 .1033 5 -.008777 .077123 .085900 -.1022 6 .079724 .093700 -.013976 -.1492 7 .069319 .090300 -.020981 -.2323 -.0971 8 .079724 .088300 -.008576 9 .136951 .140900 -.003949 -.0280 -.005475 10 .082325 .087800 -.0624 11 .079724 .083300 -.003576 -.0429 12 .074522 .072300 .002222 .0307 13 .077123 .084700 -.007577 -.0895 14 .079724 .064300 .015424 .2399 15 .082325 .061000 .021325 .3496 .079724 .070600 .009124 .1292 16

\*

#### GOODNESS-OF-FIT STATISTICS

\_\_\_\_\_ .6490 = Increase in R\*\*2 Due to Variable Added .6490 = Multiple R\*\*2.8056 = Multiple R.0114657 = Standard error of estimate (Se) .0186974 = Standard deviation of Y (Sy) .6132252 = Se/Sy.1189643 = Mean of Absolute Relative Errors .0868839 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 25.889 = Total F for the Analysis of Variance on R df 1 = 1. df 2 = 14.25.889 = Partial F to Enter df 1 = 1 df 2 = 14.

188

Var Se(bi)/bi	b	t	r	r**	:2 t*1	s Se(bi)
5 .19654	260122	8056	8056	.6490	.6490	.0511
	.243601 =	Intercept				
========	:=======	======	======	-=====	======	=======
=======		======	======		======	=======
Step numk	per = 2	Enter p	redictor	variable	1	
*****	*****	*****	****	*****	*****	*****
*****	**					

#### STATISTICAL CHARACTERISTICS

#### FOR VARIABLE SELECTION

Partial F	Partial R	
to enter	to enter	Var
17.241	7551	1
.526	1971	2
4.452	5051	3
.804	2413	4
.005	0186	6
3.353	4528	7
5.850	.5571	8

.7972 = Determinant of Intercorrelation Matrix

ERROR ANALYSIS

\_\_\_\_\_\_

Obs.	Predicted	Predicted Measured Error		Relative
No.	YP	Y	e = YP - Y	error (e/Y)
1	.070014	.072500	002486	0343
2	.066941	.071100	004159	0585
3	.064585	.070200	005615	0800
4	.063199	.069900	006701	0959
5	.083027	.085900	002873	0334
6	.088574	.093700	005126	0547
7	.080114	.090300	010186	1128
8	.086420	.088300	001880	0213
9	.137845	.140900	003055	0217

.083300

.072300

.084700

.064300

.061000

.070600

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

.087800 -.000343 -.0039

.000681

.006064

.000198

.004730

.017320

.013431

.0082

.0839

.0023

.0736

.2839

.1902

\* \* \* \* \* \* \* \* \* \* \*

10 .087457

.083981

.078364

.084898

.069030

.078320

.084031

11

12

13

14

15

#### GOODNESS-OF-FIT STATISTICS

\_\_\_\_\_ .2001 = Increase in R\*\*2 Due to Variable Added .8491 = Multiple R\*\*2 .9215 = Multiple R.0078014 = Standard error of estimate (Se) .0186974 = Standard deviation of Y (Sy) .4172458 = Se/Sy.0724124 = Mean of Absolute Relative Errors .0745621 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 36.580 = Total F for the Analysis of Variance on R df 1 = 2. df 2 = 13.17.240 = Partial F to Enter df 1 = 1 df 2 = 13.

Var Se(bi)/bi	b	t	r	r**2	t*r	Se(bi)
5 .20802	187282	5800	8056	.6490	.4673	.0390
1 .24084	000167	5010	7622	.5809	.3818	.0000
.216852 = Intercept						
=======================================						
========						
Step number = 3 Enter predictor variable 7						
****** ***	***** ***	*****	*****	*****	*****	*****

#### STATISTICAL CHARACTERISTICS

#### FOR VARIABLE SELECTION

	Partial R	Partial F
Var	to enter	to enter
2	.2402	.735
3	.4046	2.349
4	4883	3.758
6	.0311	.012
7	.6905	10.938
8	.5019	4.041

.1749 = Determinant of Intercorrelation Matrix

ERROR ANALYSIS

\_\_\_\_\_\_ Obs. Predicted Measured Error Relative ΥP No. Y e = YP - Yerror (e/Y) 1 .067535 .072500 -.004965 -.0685 2 .071100 -.0207 .069627 -.001473 3 .069550 .070200 -.000650 -.0093 4 .070300 .069900 .000400 .0057 5 -.006741 .079159 .085900 -.0785 6 .096829 .093700 .003129 .0334 7 .083614 .090300 -.006686 -.0740 .088300 8 .089792 .001492 .0169 9 .139008 .140900 -.001892 -.0134 10 .089269 .087800 .001469 .0167 -.002379 .080921 .083300 -.0286 11 12 .079372 .072300 .007072 .0978 13 .079438 .084700 -.005262 -.0621 14 .061490 .064300 -.002810 -.0437 15 .069870 .061000 .008870 .1454 .081027 .070600 .010427 .1477 16

\*

#### GOODNESS-OF-FIT STATISTICS

\_\_\_\_\_ .0719 = Increase in R\*\*2 Due to Variable Added .9211 = Multiple R\*\*2 .9597 = Multiple R.0058733 = Standard error of estimate (Se) .0186974 = Standard deviation of Y (Sy) .3141263 = Se/Sy.0539020 = Mean of Absolute Relative Errors .0457244 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 46.671 = Total F for the Analysis of Variance on R df 1 = 3. df 2 = 12.10.936 = Partial F to Enter df 1 = 1 df 2 = 12.

Var Se(bi)/bi	b	t	r	r**2	t*r	Se(bi)
5 .27864	125058	3873	8056	.6490	.3120	.0348
1 .18133	000355	-1.0636	7622	.5809	.8107	.0001
7 .30239	.012310	. 5727	3521	.1239	2016	.0037
	.154375 =	Intercept				
=======	========	.======		.======	=======	
========						
=======						
Step number = 4 Enter predictor variable 4						

#### STATISTICAL CHARACTERISTICS

#### FOR VARIABLE SELECTION

	Partial R	Partial F
Var	to enter	to enter
2	.4346	2.562
3	.4771	3.242
4	5062	3.791
6	.3496	1.531
8	.4725	3.161

.1404 = Determinant of Intercorrelation Matrix

ERROR ANALYSIS

\_\_\_\_\_\_

Obs. Predicted Measured Error Relative No. ΥP Y e = YP - Yerror (e/Y) 1 .066988 .072500 -.005512 -.0760 2 .071100 -.0191 .069742 -.001358 3 .069644 .070200 -.000556 -.0079 4 .070917 .069900 .001017 .0145 5 -.003007 -.0350 .082893 .085900 .0583 6 .099166 .093700 .005466 7 .081109 .090300 -.009191 -.1018 .088300 8 .089125 .000825 .0093 9 .139000 .140900 -.001900 -.0135 10 .089857 .087800 .002057 .0234

.083300

.072300

.084700

.064300

.061000

.070600

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

-.005904

.005816

.000405

-.000248

.005621

.006470

-.0709

.0804

.0048

-.0039

.0921

.0916

\*\*\*\*\*

11

12

13

14

15

16

.077396

.078116

.085105

.064052

.066621

\_\_\_\_\_ .0202 = Increase in R\*\*2 Due to Variable Added .9413 = Multiple R\*\*2 .9702 = Multiple R.0052904 = Standard error of estimate (Se) .0186974 = Standard deviation of Y (Sy) .2829478 = Se/Sy.0439224 = Mean of Absolute Relative Errors .0363166 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 44.090 = Total F for the Analysis of Variance on R df 1 = 4. df 2 = 11.3.790 = Partial F to Enter df 1 = 1 df 2 = 11.

Var Se(bi)/bi	b	t	r	r**2	t*r	Se(bi)
5 .30010	108406	3357	8056	.6490	. 2705	.0325
1 .17087	000341	-1.0237	7622	.5809	.7803	.0001
7 .30819	.011073	.5152	3521	.1239	1814	.0034
4 .51367	007132	1587	4530	.2052	.0719	.0037
	.171842 =	Intercept				
=======	=======	=======	=======	:=======	:=======	======
======	==					
=======		=======	=======	:======	========	======

Step number = 5 Enter predictor variable 6

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\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

#### STATISTICAL CHARACTERISTICS

## FOR VARIABLE SELECTION

	Partial R	Partial F
Var	to enter	to enter
2	.0608	.037
3	.2994	.985
6	.5953	5.489
8	.2727	.803

.0865 = Determinant of Intercorrelation Matrix

ERROR ANALYSIS

\_\_\_\_\_\_

Obs. Predicted Measured Error Relative No. ΥP Y e = YP - Yerror (e/Y) 1 .067100 .072500 -.005400 -.0745 2 .071100 .070070 -.001030 -.0145 3 .070330 .070200 .000130 .0018 4 .072964 .069900 .003064 .0438 5 -.0190 .084272 .085900 -.001628 6 .094406 .093700 .000706 .0075 7 .087185 .090300 -.003115 -.0345 8 .085952 .088300 -.002348 -.0266

.140900

.087800

.083300

.072300

.084700

.064300

.061000

.070600

-.000158

-.001352

-.006031

.005994

.002960

-.001330

.002462

.007075

-.0011

-.0154

-.0724

.0829

.0349

-.0207

.0404

.1002

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10

11

12

13

14

15

16

.140742

.086448

.077269

.078294

.087660

.062970

.063462

\_\_\_\_\_\_ .0208 = Increase in R\*\*2 Due to Variable Added .9621 = Multiple R\*\*2 .9809 = Multiple R.0044585 = Standard error of estimate (Se) .0186974 = Standard deviation of Y (Sy) .2384570 = Se/Sy.0368900 = Mean of Absolute Relative Errors .0304346 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 50.760 = Total F for the Analysis of Variance on R df 1 = 5. df 2 = 10.5.488 = Partial F to Enter df 1 = 1 df 2 = 10.

Var	b	t	r	r**2	t*r	Se(bi)
Se(bi)/b	i					
		-				
5 .56274	060694	1880	8056	.6490	.1514	.0342
1 .13661	000374	-1.1215	7622	.5809	.8548	.0001
7 .23096	.012904	.6003	3521	.1239	2114	.0030
4 .34754	009260	2061	4530	.2052	.0934	.0032
6 .42688	.084888	.1838	.4018	.1615	.0738	.0362

.100173 = Intercept

 ====

Step number = 6 Enter predictor variable 8

## STATISTICAL CHARACTERISTICS

# FOR VARIABLE SELECTION

	Partial R	Partial F	
Var	to enter	to enter	
2	1337	.164	
3	0895	.073	
8	1500	.207	
*****	******	******	********
*****	*****		

.0267 = Determinant of Intercorrelation Matrix

ERROR ANALYSIS

\_\_\_\_\_\_ Obs. Predicted Measured Error Relative No. ΥP Y e = YP - Yerror (e/Y) .072500 1 .066350 -.006150 -.0848 2 .071100 .069814 -.001286 -.0181 3 .070286 .070200 .0012 .000086 4 .073357 .069900 .003457 .0495 5 .083943 .085900 -.001957 -.0228 6 .093749 .093700 .000049 .0005 7 .087318 .090300 -.002982 -.0330 .088300 8 .086727 -.001573 -.0178 9 .140638 .140900 -.000262 -.0019 -.000580 -.0066 10 .087220 .087800 -.005700 .077600 .083300 -.0684 11 12 .077447 .072300 .005147 .0712 13 .087698 .084700 .002998 .0354 14 .063740 .064300 -.000560 -.0087 15 .062863 .061000 .001863 .0305

\*

.007451

.1055

.070600

\*\*\*\*\*\*

16

\_\_\_\_\_\_ .0009 = Increase in R\*\*2 Due to Variable Added .9629 = Multiple R\*\*2.9813 = Multiple R .0046466 = Standard error of estimate (Se) .0186974 = Standard deviation of Y (Sy) .2485146 = Se/Sy.0347506 = Mean of Absolute Relative Errors .0324285 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 38.980 = Total F for the Analysis of Variance on R df 1 = 6. df 2 = 9..207 = Partial F to Enter df 1 = 1 df 2 = 9.

Var Se(bi)/k	b oi	t	r	r**2	t*r	Se(bi)
5 1.57239	038455	1191	8056	.6490	.0959	.0605
1 .16553	000391	-1.1716	7622	.5809	.8930	.0001
7 .24988	.013504	.6282	3521	.1239	2212	.0034
4 .44143	010802	2404	4530	.2052	.1089	.0048
6 .48799	.098371	.2130	.4018	.1615	.0856	.0480
8 2.19787	000016	0525	0137	.0002	.0007	.0000

.089760 = Intercept

=======================================	 ===========	
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=======================================	 ==========	==========
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Step number = 7 Enter predictor variable 3

### STATISTICAL CHARACTERISTICS

STAT	ISTICAL CHARA	ACTERISTICS			
FOI	R VARIABLE SI	ELECTION			
	Partial R	Partial F			
Var	to enter	to enter			
2	0294	.007			
3	.2485	.527			
		******	******	*****	******
****	* * * * * *				
.00	001 = Determi	inant of Interd	correlation M	atrix	

ERROR ANALYSIS

\_\_\_\_\_\_

Obs. Predicted Measured Error Relative No. ΥP Y e = YP - Yerror (e/Y) 1 .066181 .072500 -.006319 -.0872 2 .070090 .071100 -.001010 -.0142 3 .070021 .070200 -.000179 -.0025 4 .072860 .069900 .002960 .0423 5 .083866 .085900 -.002034 -.0237 6 .094045 .093700 .000345 .0037 7 .089169 .090300 -.001131 -.0125 8 .085873 .088300 -.002427 -.0275 9 .140725 .140900 -.000175 -.0012 .0079 10 .088493 .087800 .000693 -.006868 .076432 .083300 -.0825 11 12 .076662 .072300 .004362 .0603

.084700

.064300

.061000

.070600

\*

.002202

.000205

.003010

.006365

.0260

.0032

.0493

.0902

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13

14

15

16

.086902

.064505

.064010

\_\_\_\_\_ .0023 = Increase in R\*\*2 Due to Variable Added .9652 = Multiple R\*\*2.9825 = Multiple R.0047742 = Standard error of estimate (Se) .0186974 = Standard deviation of Y (Sy) .2553403 = Se/Sy.0333893 = Mean of Absolute Relative Errors .0316204 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 31.724 = Total F for the Analysis of Variance on R df 1 = 7. df 2 = 8..525 = Partial F to Enter df 1 = 1 df 2 = 8.

Var Se(bi)/bi	b	t	r	r**2	2 t*r	Se(bi)
5 5.81301	.016930	.0524	8056	.6490	0422	.0984
1 .55991	000651	-1.9510	7622	.5809	1.4870	.0004
7 .29617	.015777	.7340	3521	.1239	2584	.0047
4 .43234	011650	2593	4530	.2052	.1175	.0050
6 .49312	.100144	.2168	.4018	.1615	.0871	.0494
8 1.23768	000126	4167	0137	.0002	.0057	.0002
3 1.37818	.000016	.6605	6531	.4265	4314	.0000

.081988 = Intercept

========
=======

Step number = 8 Enter predictor variable 2

# 

\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*

\*\*\*\*\*\*

ERROR ANALYSIS

\_\_\_\_\_\_ Obs. Predicted Measured Error Relative No. ΥP Y e = YP - Yerror (e/Y) 1 .066055 .072500 -.006445 -.0889 2 .071100 .070552 -.000548 -.0077 3 .070181 .070200 -.000019 -.0003 4 .072975 .069900 .003075 .0440 5 -.0144 .084663 .085900 -.001237 .0053 6 .094198 .093700 .000498 7 .088967 .090300 -.001333 -.0148 .088300 8 .085524 -.002776 -.0314 9 .140621 .140900 -.000279 -.0020 .0094 10 .088621 .087800 .000821 -.006594 -.0792 .076706 .083300 11 12 .075458 .072300 .003158 .0437 13 .086851 .084700 .002151 .0254 14 .063739 .064300 -.000561 -.0087 15 .064272 .061000 .003272 .0536 .077417 .070600 .006817 .0966 16

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\_\_\_\_\_ .0007 = Increase in R\*\*2 Due to Variable Added .9659 = Multiple R\*\*2.9828 = Multiple R .0050550 = Standard error of estimate (Se) .0186974 = Standard deviation of Y (Sy) .2703564 = Se/Sy.0328279 = Mean of Absolute Relative Errors .0319179 = Std. dev. of Absolute Relative Errors \*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\*\* \*\*\*\*\* 24.777 = Total F for the Analysis of Variance on R df 1 = 8. df 2 = 7..136 = Partial F to Enter df 1 = 1 df 2 = 7.

Var	b	t	r	r**2	2 t*r	Se(bi)
Se(bi)/bi	-					
5 3.37805	.033604	.1041	8056	.6490	0838	.1135
1 .68681	000793	-2.3779	7622	.5809	1.8123	.0005
7 .45008	.018193	.8464	3521	.1239	2980	.0082
4 .60906	010389	2312	4530	.2052	.1047	.0063
6 .53239	.109838	.2378	.4018	.1615	.0955	.0585
8 1.24734	000187	6184	0137	.0002	.0085	.0002
3 1.29076	.000022	.9044	6531	.4265	5907	.0000
2 2.70093	.000417	.1296	6383	.4074	0827	.0011

.061049 = Intercept